

## 6.0 LOADING, FATE, AND TRANSPORT FOR SELECT ~~INDICATOR~~ CHEMICALS~~CONTAMINANTS~~

This section presents an assessment of contaminant loading mechanisms to the Study Area from external sources as well as in-river processes affecting the concentration, transport, and fate of ~~select ICs~~ ~~select indicator-contaminants~~ within the Study Area. The evaluation provided in this section is presented in two main parts. First, Section 6.1 assesses current and historical ~~chemical-contaminant~~ inputs (i.e., external loading) to the Study Area. The loading terms/pathways discussed include upstream loading (via surface water and sediment bedload), stormwater runoff, permitted ~~point-source and non-stormwater~~ discharges (~~point-source, non-stormwater~~), upland groundwater (plume transport to river), atmospheric deposition (to the river surface), direct upland soil and riverbank erosion, groundwater advection through subsurface sediments (chemical partitioning from subsurface sediment to pore water and advection to the surface sediment interval), and overwater releases. Loading from surface sediment to the surface water via sediment erosion and resuspension (~~internal fate and transport processes~~) is not quantitatively evaluated in this RI report. Second, Section 6.2 describes ~~chemical-contaminant~~ fate and transport processes that act on contaminants in abiotic and biotic media within the Study Area. The discussion of fate and transport processes is grouped into sediment and pore water processes, surface water processes, and biotic processes.

The primary purpose of this loading assessment and discussion of relevant fate and transport processes is to support development of the CSM, which is presented in (Section-10). ~~The CSM will, in turn, support the FS in consideration of recontamination issues, monitored natural recovery, and development and evaluation of cleanup alternatives.~~ The loading assessment will also support the fate and transport modeling effort, which is being conducted in parallel with development of the RI to support the FS and will be documented in separate project deliverables.

The information presented in this section is organized by individual loading, fate, and transport mechanism. A comparative, contaminant-by-contaminant assessment of these processes is presented in the CSM ~~(Section-10)~~, along with the other informational components considered in development of the CSM, including (i.e., the observed cross-media contaminant distribution patterns, the baseline risk assessment findings, potential for human and ecological exposure, and the understanding of current and historical sources).

The ~~IC-contaminant~~ lists for this assessment are presented in Table 6.0-1. This table includes separate lists for surface water, stormwater, upland groundwater plumes, atmospheric deposition, and equilibrium partitioning (advective loading from subsurface sediment to surface sediment and from surface sediment to surface water). These lists were generated from the overall list of ~~ICs-contaminants~~ for loading, fate, and transport developed in consultation with EPA, and reflect data availability by media and relevance of the contaminant to the loading mechanism. For example, (e.g.,

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**Commented [KK1]:** This term is removed from this section. The list of indicator contaminants had changed to a consistent list for all media in Section 5. Therefore, this section just discusses select contaminants were used for this evaluation.

**Commented [awc2]:** Inserted "select" here as the quantitative evaluation is limited for some ICs and spelled out "indicator contaminants" per the convention used in Section 5. Retained INDICATOR in Section 6 title as a parallel edit.

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equilibrium partitioning ~~ICs-contaminants~~ primarily focus on hydrophobic chemicals and metals~~-, stormwater and atmospheric deposition ICs-contaminants reflect the limited available data sets-, and upland plume loading ICs-contaminants reflect individual upland plumes, etc-). Development of the loading IC-contaminants lists considered the chemical-contaminant lists for in-river nature and extent (presented in Section 5), as well as the lists for CSM presentation (Section 10) and fate and transport modeling currently under development for the FS.~~

## 6.1 EXTERNAL LOADING

This section evaluates current and historical contaminant loading to the surface sediment and surface water of the Study Area by external loading mechanisms. The boundaries of the Study Area includes the upstream and downstream river mile designations (RM 1.9 and 11.8), the surface of the river, the river bank sediment/riparian soil boundary at an elevation of +13 ft NAVD88, and the surface sediment/subsurface sediment boundary at 30 cm bml. Contaminant masses passing through these boundaries into the Study Area are external loads.

These loading mechanisms are differentiated from the “sources” described in Section 4 in that they represent the combined estimated load from all Study Area sources for the corresponding pathway. A simplified conceptualization of the external loading pathways (loading terms) to the Study Area and internal transport processes within the Study Area is presented in Figure 6.1-1. The loading terms presented on that figure were determined in consultation with EPA (Integral et al. 2006, pers. comm.), and include the following:

- Upstream loading ~~(via surface water, including suspended sediment load and sediment bedload)~~
- Stormwater runoff
- Permitted ~~non-stormwater~~ point source discharges ~~(non-stormwater)~~
- Upland groundwater ~~(plume transport to the river)~~
- Atmospheric deposition ~~(to the river surface)~~
- Upland riverbank erosion
- Groundwater advection through sediments (partitioning from deep sediment, advecting to shallow sediments)
- Overwater releases.

Numerical loading estimates were generated for all of the above terms except for upland riverbank erosion ~~(per agreement with EPA)~~, sediment bedload ~~(per agreement with EPA)~~, and overwater releases. Contaminant releases from current and/or historical overwater activities ~~(e.g., include such as sandblasting, painting, material transfer, maintenance, repair, and operations at riverside docks, wharfs, or piers)~~, discharges

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from vessels (e.g., gray, bilge, or ballast water), fuel releases, and spills are not considered quantifiable and are not addressed in this section. Releases of this nature are expected to have been more significant historically, prior to improved BMPs. While improved BMPs are likely to have reduced the occurrence of overwater releases significantly, it is acknowledged that current and future releases could occur. Due to insufficient available information, no attempt is made in this report to predict and quantify such releases as a current loading term.

This section also presents loading estimates for groundwater advection through surface sediments (partitioning from surface sediment to surface water), which is an internal loading process within the Study Area. Internal transfer mechanisms are those that involve the transport of chemical mass from one media to another within the Study Area, as defined above, but which do not add new chemical mass to the Study Area. Internal fate and transport mechanisms include sediment resuspension/transport/deposition, solid/aqueous-phase partitioning, abiotic/biotic transformation and degradation, biological uptake, and groundwater advection through surface sediments (partitioning from surface sediment to surface water). Groundwater advection is the only internal process that is quantified here. The other internal processes may be very significant in the transfer of chemicals from abiotic media and to biota, and many of these terms will be quantified as part of the FS fate and transport modeling effort.

For all loading terms for which numerical estimates were generated, a range of estimates (central estimate and upper- and lower-bounds) is provided to give perspective on the uncertainty associated with a given pathway and chemical. The estimation approach for each term varies in nature and approach from qualitative to quantitative,<sup>†</sup> depending on the degree to which loading associated with a given transport pathway could be evaluated using available information. Most, if not all, of the attributes utilized in the loading estimations are based on site-specific measurements and monitoring results. If attributes were either not measurable or site-specific data are not available, literature data or empirical data collected outside of the Study Area are used. Some assumptions and modeling or calculation techniques may be used in these assessments. The assessments of external loading terms in Section 6.1 are intended to illustrate the estimated magnitude and variability in contaminant loads to the Study Area under “typical” conditions, i.e., in an average water year.<sup>2</sup> Assessment of year-to-

**Commented [KK5]:** Concept of quantitative, semi-quantitative, and qualitative analysis has been removed. This section just discusses the approach used without these qualifiers, although the concept of quantitative and qualitative analysis are retained. The definition of quantitative, as provided by Integral, has been included into this paragraph so the reader understands the quantitative analysis used both site-specific and literature data.

<sup>†</sup>The following definitions of qualitative, semi-quantitative, and quantitative assessments are applied to this discussion:

*Qualitative*—The attributes of the analysis are not measured/measurable. Discussions consider available information and examine general trends and relative effects.

*Semi-quantitative*—Some of the attributes of the analysis are measurable while others are not. General (or literature) non-Study-Area specific data may be used in these assessments.

*Quantitative*—Most, if not all, of the attributes are measured and can be applied to the analysis. Some assumptions and modeling or calculation techniques may be used in these assessments.

<sup>2</sup> Because every water year is slightly different from the theoretical “average water year,” the analyses include data collected during a range of environmental conditions. This variability is taken into account in the analyses to the extent possible. The approach to assessment of each term is discussed in detail in Appendix E.

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year temporal variability was not the intent of this analysis.<sup>3</sup> The target scale of assessment of current loading rates is mass per year to the entire Study Area; however, in many cases, the data set supports (and calls for) calculation of loading estimates at higher temporal and/or spatial resolution. Where possible and relevant to understanding the system for the purposes of the RI, these more refined (e.g., by river mile) loading estimates were generated and are presented and discussed.

Table 6.1-1 summarizes the load estimate quantification level and calculation approach for the estimated loading terms, including the approach for generating the range of estimates. Because of data limitations for some mechanisms and chemicals, the range of estimates does not always represent the full understanding of uncertainty in the estimate; therefore Thus, additional discussion of variability and uncertainty in the estimates is provided after discussion of approach for each loading mechanism in the following subsections.

The loading rates presented in this section are estimates, and ~~They are~~ subject to varying ~~and sometimes considerable~~ degrees of uncertainty that necessarily influence their utility as lines of evidence for the CSM (~~Section 10~~). In Section 10, this information will be considered along with nature and extent information for all media, understanding of the physical system, understanding of contaminant behavior, risk information from the baseline RAs, and the understanding of current and historical sources. As such, the present section focuses on simple presentation of the loading analyses and general, mechanism-specific findings and patterns.

Subsections 6.1.1 through 6.1.7 define each current external loading term,<sup>4</sup> identify and discuss the term's target ~~IC-contaminant~~ list, ~~note its targeted level of estimation (qualitative, semi-qualitative, or quantitative)~~, describe the approach to assessment of the term's current loading rates, discuss the understanding of uncertainty in the estimates, and summarize and discuss any numerical estimates. Additional details of data sources, calculation approaches, and numerical results are presented in Appendix E. ~~It is recognized that~~ each loading term has a historical component that may be responsible for much of the observed sediment conditions in the Study Area, particularly in the subsurface. Very limited quantitative data are available to support estimates of these historical terms. Therefore, historical loading is discussed qualitatively in Section 6.1.8, ~~and further analysis is deferred to Section 10 (CSM), where the locations and pathways of historical sources are assessed relative to the surface and subsurface sediment record. Finally, a detailed comparative assessment of loading terms is provided on a chemical specific basis in Section 10 (CSM).~~

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<sup>3</sup> Extreme events are discussed for bedload in Section 6.1.1.2 to present additional information from the numerical modeling work used to generate the estimates for a typical water year. Such numerical modeling information is not available at this time to estimate extreme event loading for other loading terms.

<sup>4</sup> Due to insufficient available information, no attempt is made in this report to predict and quantify overwater releases as a current loading term.

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### 6.1.1 Upstream Loading

Upstream loading is defined as the mass rate of transport of a given contaminant into the Study Area at RM 11.8 via dissolved and particulate transport mechanisms. Upstream loading is subdivided into loading via the surface water column (dissolved and suspended solids fractions) and sediment bedload (rolling, sliding, and saltating of sediment grains). Per discussions with EPA during scoping of the ~~Fate-fate~~ and ~~Transport-transport Model-model~~ being developed for the FS, quantification of sediment bedloads into the Study Area and associated contaminant transport was deemed unnecessary because the sediment bed of the ~~Lower-lower~~ Willamette River is mainly cohesive; ~~henceHence~~, bedload transport processes are not significant components of upstream loading. Surface water loading is assessed in this section for typical flow conditions, as well as extreme observed and modeled high flow conditions.

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While upstream loading terms are presented here simply as dissolved surface water and suspended particulate loads, it is important to recognize that these loads represent the combined input to the Study Area from a variety of loading processes in the upstream watershed. These inputs include upstream point sources, upstream stormwater runoff, upstream CSOs, upstream atmospheric deposition, ~~and~~ upstream in-river sources, ~~etc.~~ Distinguishing these individual contributions to the combined upstream load is beyond the scope of this document.

#### 6.1.1.1 Empirical Surface Water Loading Estimates

~~Upstream surface water loading for a given contaminant is defined as the mass rate of transport of that contaminant into the Study Area at RM 11.8 via surface water flow. This includes both dissolved and particulate (associated with suspended solids) fractions of the chemical mass.~~ Empirical concentration and flow rate data from the site were used to generate a range of ~~quantitative~~ estimates of annual upstream surface water loading rates. Estimates for dissolved, particulate, and total loading rates are presented.

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Commented [KK10R9]: This is stated in the intro to Section 6.1.1 and does not need to be restated here.

The surface water loading ~~ICscontaminants~~ listed in Table 6.0-1 were the focus of the calculations. This ~~IC-list~~ ~~is~~ was based on a combined list of the sediment, biota, and surface water ICs designated for discussion in Section 5 regarding in-river distribution, ~~as well as~~ contaminants to be assessed in the Fate and Transport Model ~~under development~~ for the FS and the CSM (~~Section 10~~). Chemicals from this combined set of ~~ICs-contaminants~~ that were not analyzed in the RI surface water sampling program were removed from the final surface water ~~IC~~-list.

Upstream surface water loading at RM 11.8 was estimated based on the combined analytical data collected from the two nearest sampling transects: RM 16 and 11. In addition to upstream surface water loading at RM 11.8, loading rates were generated at the other RI surface water sample transect locations in the ~~Lower~~ Willamette River: RM 16, 11, 6.3, 4, and 2. Estimated loads are also presented for the transect located in Multnomah Channel, near its upstream connection with the Willamette River at RM 3. These loading estimates are not representative of upstream loading, but were generated

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to provide additional insight into the nature of the contaminant load transported by surface water as it moves through and out of the Study Area. The sum of the estimated loads at Multnomah Channel and RM 2 represents the estimated load of contaminants leaving the Study Area.<sup>5</sup>

The following subsections provide a summary of the data sets and approach used in the calculations, as well as a presentation and discussion of the findings. Detailed presentations of the data sets, data treatment, calculations, assumptions, and results can be found in the supporting Appendix E [Section 2.0](#).

#### 6.1.1.1.1 Data Sets and Approach

Surface water loading rates were estimated based on Round 2A and 3A surface water chemical concentration data from transect sampling locations and USGS flow information from RM 12.8 (Morrison Bridge Station 14211720). [To differentiate surface water loads associated with high-flow and low-flow conditions during a typical flow year,](#)<sup>6</sup> [the first step in the analysis was to determine the fraction of a typical water year that is described by each flow regime.](#) Three surface water sampling events from the Round 2A sampling effort and four surface water sampling events from the Round 3A sampling effort provided the analytical data for the surface water loading calculations. Of these seven sampling events, four occurred during low-flow conditions (<50,000 cfs), two were during high-flow conditions (>50,000 cfs), and one was during a low-flow stormwater event (active runoff to the Study Area with river flow rate <50,000 cfs). Average discharge rates (recorded as cfs) for each event are based on 30-minute measurements collected by the USGS at the stream flow station located upstream of the Morrison Bridge at RM 12.8 (Station 14211720).<sup>7</sup> Total flow volumes and high-flow:low-flow volume fractions for the individual years and the 28-year average are presented in Table E2-1. Because the data sets compared well, the 28-year hydrograph was considered adequately representative for use as the basis for defining the high-flow:low-flow volume ratio for a typical year. Fifty-two percent of the total annual volume occurred during high-flow conditions and 48 percent during low-flow conditions, for a volume ratio of 1.07. The average annual duration of the low-flow period is 268 days, while high flows occur for a much shorter period of 98 days. [Accordingly, a ratio of 52:48 was used to apportion the fractions of the typical annual hydrograph assigned as high flow to that assigned as low flow. This step is described in](#)

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<sup>5</sup> Due to the complicating influence of water from the Columbia River on Willamette River flow volume and direction at the RM 2 and Multnomah Channel transect locations (described in Section 5.3), modeled flow data were used to estimate loads for these two transects. The load calculation approach is discussed in detail in Appendix E [Section 2](#).

<sup>6</sup>~~The analysis of the surface water data presented in Section 5.3, which broke down the data set by high- and low-flow conditions, yielded generally consistent concentration patterns and supported interpretation of results. Therefore, the loading analysis perpetuated this distinction to maintain this additional level of detail (as opposed to simply presenting annual loads) in the assessment of the surface water loading estimates.~~

<sup>7</sup> Note: The flow rate values presented here are daily mean stream flow measurements from the USGS National Water Information System, [www.waterdata.usgs.gov](http://www.waterdata.usgs.gov). These values were taken from the USGS Web site on June 16, 2008, and are considered to be draft and subject to change by USGS, which may refine ratings and calculations as needed.

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detail in Appendix E Section 2.2.1. The Round 2A and 3A surface water sampling events and daily hydrograph data for the years characterized by those events (2004 through 2007) are presented in Figures 5.3-2 through 5.3-5. Surface water sampling events superimposed on the 36-year average annual hydrograph are presented in Figure 5.3-1.

The distribution of Willamette River flows between Multnomah Channel and RM 2 were based on the results from the EFDC physical transport modeling effort (WEST 2006a). The results of this model provided the average daily flow rates at Multnomah Channel, RM 2, and upstream transects (modeled upstream transect flows were used for comparison purposes only). A detailed description of the use of these model flow results is presented in Appendix E Section 2.2.1. The surface water data include particulate and dissolved chemical concentrations.<sup>8</sup> Loading estimates for these fractions were combined to generate the total surface water loading estimates.

~~To differentiate surface water loads associated with high flow and low flow conditions during a typical flow year,<sup>9</sup> the first step in the analysis was to determine the fraction of a typical water year that is described by each flow regime. To do this, the average annual hydrograph (based on the 28 year period of record from 1975 to 2003) for the Study Area was evaluated relative to a 50,000 cfs threshold for low flow versus high flow conditions. Based on this, the average annual duration of the low flow period is 268 days, while high flows occur for a much shorter period of 98 days. In terms of total annual flow volume, 52 percent is associated with the high flow periods and 48 percent with the low flow periods. Accordingly, a ratio of 52:48 was used to apportion the fractions of the typical annual hydrograph assigned as high flow to that assigned as low flow. This step is described in detail in Appendix E 2.2.1.~~

Next, the surface water analytical data set was used to estimate representative concentration ranges for high-flow and low-flow conditions for each transect. This calculation step, described in detail in Appendix E Section 2.2.2, involved ~~“subaveraging”~~ averaging individual data points associated with a transect (e.g., NB, NS, east, middle, west, etc.) to resolve the data set to one concentration estimate per transect per sampling event. Using ~~the these subaveraged data results~~, a set of minimum, mean, and maximum measured concentrations at each transect was generated for both high-flow and low-flow conditions.

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**Commented [KK14R13]:** See response above.

**Commented [awc15]:** Retain “subaveraging” terminology to avoid needing to make edits to other sections. In particular, App E uses term “subaveraging” throughout.

**Commented [KK16R15]:** No. Need to replace “subaverage” with “average” in Appendix E as well.

<sup>8</sup> A comparison of surface water particulate concentrations and physical characteristics of suspended sediment to sediment trap results was conducted to assess the adequacy of the surface water information for describing an entire water year. This assessment is presented in Appendix E Section 2. The assessment indicated that the ranges in surface water suspended sediment concentration generally capture the range observed in the sediment trap data set.

<sup>9</sup> ~~The analysis of the surface water data presented in Section 5.3, which broke down the data set by high and low flow conditions, yielded generally consistent concentration patterns and supported interpretation of results. Therefore, the loading analysis perpetuated this distinction to maintain this additional level of detail (as opposed to simply presenting annual loads) in the assessment of the surface water loading estimates.~~

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Loading rates at each transect were then estimated as the product of the chemical concentrations and the flow volumes associated with the high-flow and low-flow portions of the hydrograph. For each transect, lower, central, and upper estimates of high-flow loading were estimated by multiplying the minimum, mean, and maximum concentrations, respectively, by the total annual flow volume estimated for high-flow conditions. Likewise, a range of low-flow rate loading estimates for each transect was estimated using the minimum, mean, and maximum of subaveraged concentrations and the estimated flow volume for low-flow conditions. The range of annual mass loading rate estimates were, in turn, generated by summing the fractional loading contributions estimated for high-flow and low-flow conditions at the given transect (i.e., the total annual central flow estimate was calculated as the sum of the central high-flow estimate and the central-low flow estimate).

There are no surface water sample results available from RM 11.8, which defines the upstream boundary of the Study Area. Therefore, high-flow and low-flow concentrations at RM 11.8 were estimated by combining data from RM 16 with selected data from RM 11. Because some of the surface water samples collected at RM 11 appear to have been influenced by one or more source areas of ~~IC~~selected contaminants between RM 11.8 and RM 11, some of the RM 11 results are not representative of water quality at RM 11.8. Prior to combining the data, the RM 11 data set was assessed for each ~~IC~~selected contaminant to determine whether the data represented the same population of upstream data as that sampled at RM 16. This approach assumes that the surface water concentrations at ~~RM-RM~~ 11.8 would be more similar to those at RM 16 than those at RM 11, recognizing that although there are additional sources between RM 11.8 and 16, the proximity of likely sources to the RM 11 transect are expected to have the larger effect. A comprehensive graphical and statistical comparison of the chemical data from RM 16 and 11 was conducted for each ~~IC~~selected contaminant.<sup>10</sup> This involved plotting and evaluating data, running Mann-Whitney U statistical tests, and removing RM 11 sample results from the combined set where the evidence indicated the potential influence of a local source area. This analysis and all figures and statistical results are presented in detail in Appendix E Section 2.2.2.1.

#### 6.1.1.1.2 Uncertainty

Uncertainty associated with the surface water loading estimates is related primarily to the adequacy and representativeness of the analytical data set. The data sets are derived from grab samples, not time-weighted composites. Further, a limited number of samples were collected under a limited number of flow conditions. This prohibits a thorough understanding of temporal and flow variability in surface water quality and is an important source of uncertainty. The magnitude and direction (~~i.e., potential positive or negative~~) bias on loading estimates is unknown. Finally, additional uncertainty is associated with the RM 2 and Multnomah Channel loading results stemming from the

<sup>10</sup> Data were available for RM 11 and RM 16 for all ~~IC~~selected contaminants except BEHP, which was only sampled at RM 11. For this chemical, upstream loads were calculated based on the RM 11 sampling results.

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use of modeled flow rates and the variable influence of the Columbia River in the lower reaches of the Study Area.

Regarding uncertainty in the suspended solids component of the surface water loading estimates, sediment trap data provide some additional perspective regarding uncertainty in the suspended solids component of the surface water loading estimates because they reflect longer sampling periods (four quarterly samples) ~~however~~. However, for direct comparison, these samples are spatially limited (not representative of an entire transect) and mechanism-limited (not necessarily likely to equally capture particles of all sizes). The sediment trap results are discussed in comparison to surface water TSS in Appendix E, Section 2.4.1. This comparison suggests that the surface water TSS data reasonably approximate the longer-term sediment concentrations provided by the sediment traps, in spite of the snapshot nature of the surface water samples, lending some confidence to the use of the TSS data in the loading estimates.

#### 6.1.1.1.3 Findings

Upstream (RM 11.8) surface water loading estimates are presented on an annualized basis for all surface water loading ~~IC~~ contaminants and are further differentiated for high-flow and low-flow periods of the annual hydrograph, as well as for the particulate and dissolved fractions. Surface water loading estimates at RM 16, 11, 6.3, 4, Multnomah Channel (RM 3), and RM 2 are also presented for select indicator ~~chemicals~~ contaminants—PCBs, PCDD/Fs, total DDx, and PAHs—to provide insight into patterns of mass transport of these chemicals in surface water. Loading estimates for all transects for the surface water ~~IC~~ contaminants are presented in Appendix E, Section 2.3.

#### Annual Upstream Loading

Table 6.1-2 presents the range of total (dissolved plus particulate) annual upstream loading estimates (RM 11.8) for each surface water loading ~~IC~~ contaminant selected. The following paragraphs discuss the findings for each ~~chemical~~ contaminant (or ~~chemical~~ contaminant group) regarding patterns in total loading rate estimates, loads as a function of surface water flow regime (high flow versus low flow), and particulate-dissolved components of the loads.

Figure 6.1-2 presents total surface water loading estimates for total PCB congeners, selected individual PCB congeners, and PCB TEQ. Both the total PCBs and PCB TEQ loading estimates show higher aggregate loads during the ~~268-day~~ low-flow period of the year ~~(based on the 28-year average annual hydrograph)~~, as compared to the ~~98-day~~ high-flow period. On a daily basis, total PCB loads are higher during the high flow period than during the low-flow period. Total PCBs and PCB TEQ show significant contributions of particulate-associated concentrations to the total surface water PCB loads for most flow conditions (Figure 6.1-3), which is expected given the highly

hydrophobic nature of PCBs.<sup>11</sup> These patterns in flow conditions and particulate/dissolved ratios are also generally apparent in the individual congener data sets. Total PCBs 81, 126, and 169 exhibited low detection frequency in the upstream surface water loading data set (19 percent, 40 percent, and 14 percent, respectively; see Table E2-5 in Appendix E); therefore, the loading estimates presented for these analytes are considered more uncertain than those for other ~~ICs~~contaminants with higher detection frequency.

Total PCDD/Fs and TCDD TEQ (Figures 6.1-4 and 6.1-5) are primarily associated with particulate matter, which is also expected given the highly hydrophobic nature of these chemicals. ~~For both total PCDD/Fs and TCDD TEQ, neither the aggregate 268-day low-flow load estimates nor the aggregate 98-day high-flow load estimates dominate the total annual loading for both total PCDD/Fs and TCDD TEQ. Instead,~~ the relative contributions to the annual load from high-flow and low-flow periods are comparable (Figure 6.1-4). On a daily basis, total PCDD/Fs and TCDD TEQ loads are higher during the high-flow period than during the low-flow period.

The results for DDx compounds (Figures 6.1-6 and 6.1-7) indicate a different pattern than those noted for PCBs and PCDD/Fs. While the loads are generally higher in the particulate fraction, as typically seen for the other strongly hydrophobic ~~ICs~~contaminants, the patterns with surface water flow regime differ. For DDx compounds, the annual aggregate upstream load associated with the 98-day high-flow period is consistently higher than that associated with the 268-day low-flow period of the year ~~(based on the 28-year average annual hydrograph)~~. On a daily basis, total DDx loads are higher during the high-flow period than during the low-flow period. Further, the 4,4'-isomers of the DDx components compose the majority of the total DDx upstream load, with DDT isomers being the greatest fraction, and DDD isomers being the smallest fraction of the total DDx.

Upstream surface water loads of total PAHs are significantly higher (approximately an order of magnitude) than total cPAHs (Figure 6.1-8). This indicates that PAHs in the water column are dominated by LPAHs rather than HPAHs, as the total cPAH sum includes the majority of HPAH compounds. This pattern is also reflected in loads for naphthalene (an LPAH) as compared to BaP (an HPAH). ~~The cPAH BaPEq trends parallel total cPAHs and BaP trends.~~ For both LPAHs and HPAHs, the annual aggregate load associated with the 98-day high-flow period is higher than that associated with the 268-day low-flow period of the year ~~(based on the 28-year average annual hydrograph)~~. On a daily basis, total PAH loads are higher during the high-flow period than during the low-flow period. The LPAHs (naphthalene), which have higher solubility than HPAHs, show higher fractions of dissolved as compared to particulate

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<sup>11</sup> In surface water, partitioning theory (often discussed in the context of groundwater or pore water) still holds; however, the total chemical mass in the dissolved state in surface water can be comparable to or greater than the total chemical mass in the sorbed state, even for very hydrophobic chemicals. This is because the mass ratio of water to solids (suspended) in surface water is much higher than in groundwater or sediment.

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load (Figure 6.1-9). In contrast, the HPAHs (cPAHs, ~~cPAH-BaPEq~~, and ~~BAPBaP~~), which are more hydrophobic, show higher fractions of particulate as compared to dissolved load. It is important to note that total naphthalene exhibited low ~~(10 percent; see Table E2-5 in Appendix E)~~ detection frequency in the upstream surface water loading data set ~~(10 percent; see Table E2-5 in Appendix E)~~. Therefore, the loading estimates presented for ~~this analyte naphthalene~~ are considered more uncertain than those for other ~~ICs contaminants~~ with higher detection frequency.

**Commented [awc18]:** Retain "BAP" and "DRH" terminology to avoid needing to edit to other sections and data products. A change from BAP to BaP and from DRH to TPH (diesel) ripples through numerous figures and tables (Sec. 5, 6, and 10).

**Commented [KK19R18]:** No, this is the correct convention. These terms are not used in any other section of the RI other than the risk assessments, which use the correct convention.

Upstream loading rate ranges for BEHP and hexachlorobenzene are presented on Figures 6.1-10 and 6.1-11. The total annual BEHP load is almost exclusively associated with high-flow periods of the hydrograph, with three high-flow samples measured above detection limits and one detected low-flow sample. Total BEHP exhibited low ~~(19 percent; see Table E2-5 in Appendix E)~~ detection frequency in the upstream surface water loading data set ~~(19 percent; see Table E2-5 in Appendix E)~~. Therefore, the loading estimates presented for ~~this analyte BEHP~~ are considered more uncertain than those for other ~~ICs contaminants~~ with higher detection frequency. ~~The high-flow contribution For for hexachlorobenzene, the high flow contribution~~ is also higher than the low-flow contribution, but only by approximately 15 percent for the central estimate. Hexachlorobenzene shows consistent fractions of particulate and dissolved contributions to the total load under all flow conditions, with the particulate fraction making up roughly 15 to 20 percent of the total load.

Upstream surface water loading rate estimates for the indicator non-DDx pesticides are presented on Figures 6.1-12 and 6.1-13. These figures show that, ~~of these compounds~~, dieldrin exhibits the highest annual upstream loads, whereas aldrin loads are comparatively very low. This difference may reflect the fact that aldrin, ~~in comparison to dieldrin~~, degrades relatively rapidly in surface water by photochemical or microbial processes (discussed further in Section 6.2). Similar to the DDx pesticides discussed previously, ~~all most of the non-DDx pesticides show exhibit~~ higher loads during high-flow conditions, with the exception of gamma-HCH, which ~~shows exhibits~~ approximately 60 percent of the annual load during low-flow conditions. In contrast to the DDx pesticides, ~~however~~, the total surface water loads for the non-DDx pesticides are dominated by the dissolved fraction (Figure 6.1-13).

Figures 6.1-14 and 6.1-15 present the upstream surface water loading rate estimates for the indicator metals. The highest overall loading rates are observed for zinc and copper, two common elements. Nickel and chromium loads are the next highest and ~~show exhibit~~ similar loading rate estimates. Lead and arsenic ~~show exhibit~~ the next highest loading estimates, with mercury loads being the lowest and the result of only a few detections. Total mercury exhibited low ~~(23 percent; see Table E2-5 in Appendix E)~~ detection frequency ~~(23 percent; see Table E2-5 in Appendix E)~~. Therefore, the loading estimates presented for this analyte are considered more uncertain than those for other ~~ICs contaminants~~ with higher detection frequency. ~~For all of these metals~~, loading rates during high-flow conditions ~~for all of these metals~~ are greater than loading rates during low-flow conditions. Further, ~~for the majority of the metals~~, the particulate

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fraction contributes more than the dissolved fraction to the total loading estimates for the majority of the metals, especially under high-flow conditions (Figure 6.1-15).

Estimated upstream total surface water loads for TBT are presented on Figure 6.1-16. ~~(There is no presentation of dissolved versus particulate fractions for TBT because the surface water data set includes measurements of total concentrations only.)~~ TBT in upstream surface water was detected only once, and that was during a low-flow sampling event (high-flow results were below detection limits); ~~therefore~~ Therefore, the loading estimates for low flow represent 100 percent of the annual load estimates (the load associated with the high flow period was estimated to be zero per loading calculation data rules; i.e., non-detect results were set to zero). Generally, due to the low detection frequency (5 percent; see Table E2-5 in Appendix E) in the upstream surface water loading data set, the loading estimates presented for total TBT are considered more uncertain than those for other ICs with higher detection frequency; no meaningful conclusions/comparisons could be drawn/made regarding the relative loading of TBT with regard to low flow versus high flow conditions.

**Commented [awc20]:** Retain the first part of the deleted text ("the loading estimates for low flow represent 100 percent of the annual load estimates") because it provides meaningful information to the reader.

**Commented [KK21R20]:** The deleted text repeats what is stated in the previous sentence. It does not add any meaningful information.

In summary, with the exception of PCBs and gamma-HCH, regarding the relationships of loading to flow conditions, all of the surface water ICs/contaminants, with the exception of PCBs and gamma-HCH, show/exhibit higher upstream loading rates during high-flow conditions than during low-flow conditions; ~~when aggregated over the 98-day high flow period and the 268-day low flow period (based on the 28-year average annual hydrograph).~~ On a daily basis, loads for all of the ICs/contaminants are generally higher on a daily basis during high flows than during low flows. Regarding overall patterns in dissolved and particulate fractions of total upstream loads, ~~(The~~ particulate fraction represents the larger component for PCBs, PCDD/Fs, DDx pesticides, and metals. The dissolved fraction is the larger component for LPAHs, non-DDx pesticides, and hexachlorobenzene. In general, the ratios of particulate to dissolved mass loading for all surface water loading ICs/contaminants do not show large or consistent variations under different flow conditions (high flow versus low flow), indicating possible ~~possibly suggesting~~ conditions of equilibrium or near equilibrium ~~as discussed further in Section 6.2.2).~~

#### Surface Water Mass Transport within the Study Area

To evaluate spatial trends in surface water loading estimates moving downstream through the Study Area, loading estimates were plotted for each available transect for total PCBs, total PCDD/Fs, TCDD TEQ, total DDx, and total PAHs. These plots are presented in Figures 6.1-17 through 6.1-19. As mentioned above in the approach discussion, upstream loads at the RM 11.8 Study Area boundary were estimated based on combining RM 11 and RM 16 data. Further, the fraction of the Willamette River load leaving the Study Area at RM 2 and Multnomah Channel was estimated based on measured concentrations and modeled river flows, due to the hydrologically complex influence of water from the Columbia River on flows in the lower Willamette River. These transect estimates are distinguished with blue (RM 11.8) and orange (RM 2 and Multnomah Channel) symbols on Figures 6.1-17 through 6.1-19. The sum of the

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estimated loads at Multnomah Channel and RM 2 represents the estimated load of chemicals leaving the Study Area.

The total PCB plots (Figure 6.1-17) show that central estimates of surface water loading increase continually from RM 16 to 4. The upper and lower estimates show the same increasing trend. Comparison of the central estimate results from the upstream estimate (RM 11.8) and the from the Multnomah Channel and RM 2 transects indicate that the total PCB load leaving the Study Area is estimated to be greater than the upstream end of the Study Area. This observation of a loading increase from upstream to downstream is supported by the observed concentration increase between RM 11 and RM 4 in six of seven surface water sampling events, as discussed in Section 5.4.3-6.

Total PCDD/Fs and TCDD TEQ loads show patterns similar to PCBs (Figure 6.1-18), with increasing loads moving downstream through the Study Area; however the increase is not as steady, with comparable central loading estimates at RM 11 and 6.3. Comparison of the central estimate results from for RM 11.8 with the Multnomah Channel and RM 2 transects indicates that the total PCDD/F and TCDD TEQ loads leaving the Study Area are estimated to be greater than the upstream end of the Study Area. However, the RM 11 transect loads are comparable to the summed RM 2 and Multnomah Channel loads, suggesting that much of this loading increase occurs between RM 11.8 and 11. Consistent with the loading observations, total PCDD/F concentrations in surface water do not display consistent trends from upstream to downstream within the Study Area, as discussed in Section 5.4.5-7.

Total DDx pesticides (Figure 6.1-19, top plot) show fairly consistent loads between RM 16 and 11, with loads increasing in the Study Area between RM 11 and 4. Finally, total PAHs (Figure 6.1-19, bottom plot) show a continually increasing load between RM 16 and 4. As with PCBs and PCDD/Fs, the central estimates of DDx and PAH loads leaving the Study Area at Multnomah Channel and RM 2 are somewhat higher than the loads estimated at the upstream end of the Study Area (RM 11.8). For both DDx and PAHs, this observation of a loading increase from upstream to downstream is supported by the observed concentration increase between RM 11 and 4 in six of seven surface water sampling events, as discussed in Section 5.4.6-8 (DDx) and 5.4.7-9 (PAHs).

A graphical summary comparison of loading terms is presented in Section 10.2 for the 13 CSM chemicals.

#### 6.1.1.2 Modeled Sediment Fluxes

The 2009 ~~FS~~-HST model was used to evaluate the relative scale of the sediment fluxes to the Study Area over a range of flow conditions. As discussed previously, bedload is not quantified in the ~~FS~~-HST model because the physical CSM for the Lower Willamette River assumes that bedload represents a relatively small fraction of the total sediment load entering the Study Area at RM 11.8. This is because of the Lower Willamette River's morphology and the fact that its flows are regulated by upstream

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control structures. As noted in Section 3, the Study Area occupies the lower portion of the Lower Willamette River where the river widens and has been deepened by dredging. The reach upstream of the Study Area, from Willamette Falls through downtown Portland, is generally narrower and faster moving, so suspended loads tend to be transported into the Study Area before settling out. In addition, dams at Willamette Falls (RM 26) and further upstream trap bedload moving downstream from the middle Willamette River to the Lower Willamette River. The only significant tributary to the Lower Willamette River below RM 26, the Clackamas River at RM 24.7, is a gravel-bed stream. Much of the bedload from the Clackamas River is likely deposited in the Willamette River well upstream of the Study Area—~~e.g.,~~ the main stem of the Lower Willamette River just upstream and at Ross Island has several large depressions in the channel (see Map 3.1-6) that trap bedload materials moving downstream.<sup>12</sup> From a contaminant loading perspective, the significance of bedload is further reduced because this process transports relatively coarse-grained, non-cohesive, clastic sediments, which have a generally low organic carbon content and low surface-to-volume ratio. Therefore, associated contaminant concentrations are presumably much lower than concentrations associated with finer grained suspended sediment.

#### 6.1.1.2.1 Modeled Sediment Flux

The ~~2009 FS~~ HST model was used to predict sediment loads entering the Study Area from upstream at RM 11.8 under five different flow regimes. The flow regimes range from the 5<sup>th</sup> to 95<sup>th</sup> percentiles of flow conditions based on the Lower Willamette River flow data recorded since 1972. The specific flow years modeled (starting on October 1 and running through September 30 of the year indicated) and the mean annual flows for those years are listed below:

- 5<sup>th</sup> Percentile Flow Year 2001 – mean flow 454 m<sup>3</sup>/sec
- 25<sup>th</sup> Percentile Flow Year 1981 – mean flow 787 m<sup>3</sup>/sec
- 50<sup>th</sup> Percentile Flow Year 1986 – mean flow 878 m<sup>3</sup>/sec
- 75<sup>th</sup> Percentile Flow Year 1995 – mean flow 1,078 m<sup>3</sup>/sec
- 95<sup>th</sup> Percentile Flow Year 1997 – mean flow 1,522 m<sup>3</sup>/sec.

These flow year percentiles were developed for use in the fate and transport modeling effort. ~~The Fate and Transport Model is being developed~~ to support evaluations of contaminant fate, transport, and loading for the RI and FS. As noted above, the fate and transport modeling effort is ongoing and the results will be evaluated and reported separately.

The ~~FS~~ HST model estimates the daily-averaged flux of water and suspended sediment (cohesive and non-cohesive). Table 6.1-3 and Figure 6.1-20 (Petroni 2011, pers.

<sup>12</sup> As discussed in Section 5.6, there are also several man-made borrow pits spread across the channel in the upper Study Area between RM 10 and 11, which also likely entrain bedload entering the Study Area.

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comm.) summarize the modeled suspended sediment fluxes for each flow regime entering the Study Area at RM 11.8 and exiting the Study Area in the main stem of the Lower Willamette River at RM 1.2<sup>13</sup> and in Multnomah Channel. Negative values in Table 6.1-3 indicate material leaving the Study Area. The averaged annual modeled flow estimates illustrate the relative percent increase in discharge out the Multnomah Channel at lower Lower Willamette River flow levels. The lower portion of Table 6.1-3 shows the combined sediment fluxes exiting the harbor at RM 1.2 and in the Multnomah Channel and then tallies the modeled suspended and total sediment loads entering and leaving Portland Harbor for each flow regime.

The net flux of suspended sediment for all flow regimes modeled is positive, indicating that the harbor is generally a trap for material entering from upstream. Across all flow regimes, an average of 82 percent of the total modeled sediment flux entering the harbor at RM 11.8 passes through and exits at RM 1.2 and the Multnomah Channel. The 18 percent retained within the Study Area represents an estimated average annual net sediment accumulation of 277 million kg/year and reflects the overall net depositional character of the harbor Study Area. However, as detailed in Section 3.1.5.2.6 and summarized in Table 3.1-5, on a more localized scale, various subareas within of the Study Area may be largely depositional, erosional, or subject to both processes.

Commented [awc22]: We point the reader to the discussion in Section 3 that provides details the support this statement.

Commented [KK23R22]: OK as modified.

### 6.1.2 Stormwater Runoff

This section presents the results of the calculated stormwater loading to the Study Area using stormwater and outfall sediment trap data collected as part of the RI/FS.

The chemicals-contaminants listed in Table 6.0-1 as stormwater loading ICs-contaminants were the focus of the calculations described in this section. This list e stormwater ICs-were was derived from the sediment, biota, and surface water ICs-contaminants designated for fate and transport evaluation using the Fate and Transport Model and for discussion in the RI CSM. The stormwater IC list, and was further reduced based on analyte-availability in the stormwater data set.

Estimated stormwater loads were generated for each IC-contaminant and Fate-fate and Transport-transport Model-model cell and for the entire Study Area. The following subsections present a summary of the data sets and approach used in the calculations, as well as a presentation and discussion of the findings.

Appendix E Section 3.0 describes the detailed steps taken to calculate stormwater loading estimates, from data sources/data treatment to calculation approach. Appendix E3.0 also presents the complete results, as and well as a brief discussion of the associated uncertainty. The results are summarized here and also discussed in Section 10.

<sup>13</sup> The Study Area boundary is RM 1.9 but the FS-HST model grid extends downstream to RM 1.2. This model boundary, as well as the downstream end of the Multnomah Channel model cell, were used as the downstream boundaries for these calculations.

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#### 6.1.2.1 Data Sources and Calculation Approach

The stormwater composite water and sediment trap data were collected in accordance with the Round 3A Stormwater FSP and Addendum (Anchor and Integral 2007a,b) and its companion document, the Round 3A Stormwater Sampling Rationale (Anchor and Integral 2007c) and analyzed in accordance with the QAPP Addendum 8 (Integral 2007b).

The stormwater sampling location rationale was developed in accordance with a commonly used approach of applying “representative” estimates of stormwater chemical concentrations for various land use types (Scheuler 1987). A land-use-based chemical load modeling approach was used to estimate loads across the entire Study Area. Chemical loading models use site characteristics (e.g., land use and percent impervious area) and land-use-specific loading rates to estimate overall loading into the receiving waters. This approach has been modified to better fit the ~~unique~~ data needs and land use characteristics of the Study Area, as well as the practical constraints for this sampling effort.

Loads to the Study Area are calculated based on composite water and sediment trap data collected from heavy industrial, light industrial, residential, parks/open space, major transportation, and non-representative locations. Non-representative sites are those sites with non-representative chemical sources that cannot be easily extrapolated from generalized land use measurements. Twenty-seven stormwater outfalls were sampled within the Study Area to estimate stormwater loads. In general, three to five composite water samples and one sediment trap sample were collected at each stormwater sample site. As discussed in Appendix E Section 3.5.1, pesticides were analyzed at a small subset of locations (8 stations) in composite water samples, but they were analyzed at nearly all locations (22 stations) in sediment trap samples. Due to the lack of representative composite water samples for pesticides, sediment trap data and the resulting statistics (e.g., central tendencies) used in loading estimates were substituted for composite water statistics for light industrial, parks/open space, residential, and transportation land uses, as well as for 1 of 3 non-representative locations that did not have composite water data (WR-147). Additionally, composite water data and the resulting statistics used in loading estimates were substituted for sediment trap statistics for 2 of 3 non-representative locations that did not have sediment trap data (OF-22B and WR-96). For most non-pesticide chemicals, load estimates ~~to for composite~~ water are based on roughly 100 stormwater samples across all land uses and sites. For these non-pesticide chemicals, the ranges of numbers of composite water samples available for loading estimates were 27 to 72 for heavy industrial, 10 to 16 for light industrial, 9 to 10 for residential, 2 to 3 for parks/open space, 7 to 9 for major transportation, and 3 to 5 for each non-representative location. For pesticides, load estimates are based on approximately 26 composite water samples (from 8 stations) and about 19 sediment trap samples (from 19 stations) across all land uses and sites. For the pesticides, the ranges of numbers of composite water samples available for loading estimates were 12 for heavy industrial, 4 for light industrial (from one station), 3 for residential (from 1

Commented [KK24]: New edit based on discussions between EPA and LWG on June 3 and 6.

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station), zero for parks/open space, zero for major transportation, and 3 to 4 for each non-representative location. (Note that composite sample water data were only used for loading estimates for the heavy industrial and non-representative locations.) The numbers of sediment trap samples available for pesticide loading estimates were 11 for heavy industrial, 3 for light industrial, 2 for residential, 1 for parks/open space, and 1 for major transportation, and 1 for non-representative locations.

As detailed in Section 4.4.1.2 and Appendix E Section 3.0, “representative” samples from five general categories of land use (heavy industrial, light industrial, residential, major transportation, and parks/open space), as well as samples from non-representative locations, were included to obtain a practicable and sufficient data set to estimate stormwater loading to the Study Area. ~~Non-representative sites are those sites with non-representative chemical sources that cannot be easily extrapolated from generalized land use measurements.~~ Samples were collected from a subset of drainage basins/outfalls within each land use category in the Study Area. These locations were sampled by LWG and Port of Portland (Terminal 4) during two sampling efforts in the spring/summer of 2007 (Round 3A) and the fall/winter of 2007-2008 (Round 3B). One additional site (GE Decommissioning) was sampled by GE during the same time frame. Results from the GE investigation are also included in the overall LWG stormwater data set. In early 2008, the City of Portland collected three additional samples to supplement the residential data set, and these samples are included as well.

**Commented [awc25]:** Request that quotation marks be retained on the term “non-representative”. Removal of the quotation marks suggests that the data is meaningless, when in fact the term “non-representative” is basically shorthand for “non-representative of the five specifically-analyzed land use categories”.

**Commented [KK26R25]:** No. Definition has been moved to previous paragraph to clarify the meaning of non-representative so quotations are not necessary.

As a first step, the stormwater analytical data set was used to generate concentration ranges for each land use and non-representative site. Both stormwater composite water chemistry ~~composite water~~ samples and sediment trap chemistry data were used to provide two independent means of estimating stormwater chemical-contaminant loads.

Next, the stormwater runoff volumes draining to each Fate and Transport Model cell were calculated for each land use and non-representative location using the City of Portland’s GRID model as summarized in Appendix E3.5.3. It was not possible to develop runoff volumes and stormwater load estimates for individual outfalls due to uncertainty of stormwater basin boundaries for many outfalls.

As explained in Appendix E Sections 3.5.1 and ~~E~~3.5.2, loads were then estimated as a product of the calculated concentration estimates and the flow rate from the 50<sup>th</sup> percentile flow year to represent the central tendency (CT) of flow conditions. The annual mass loads were generated by adding the loading contributions from each land use and non-representative site for each Fate and Transport Model segment:

~~Annual stormwater chemical load (kg/yr) = heavy industrial stormwater chemical load (kg/yr) + light industrial stormwater chemical load (kg/yr) + residential stormwater chemical load (kg/yr) + parks/open space stormwater chemical load (kg/yr) + major transportation stormwater chemical load (kg/yr) + non-representative site stormwater chemical load (kg/yr).~~

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The process for estimating stormwater loads, as well as all figures, maps, and statistical results, is presented in detail in Appendix E [Section 3.0](#).

#### 6.1.2.2 Uncertainty

The primary sources of uncertainty in the stormwater loading estimates are the sample size and sampling period extrapolated to represent the composite conditions of a typical water year over the entire [Lower Willamette River](#) runoff area. Specifically, data used to estimate the stormwater loads were collected during a total of 15 storm events, with each outfall sampled an average of three times. Sediment traps were left in place for three to seven months during two separate sampling periods. Due to the limited time span of sampling and the known variability of stormwater, these data should be considered to represent a “snapshot” of stormwater entering the Study Area during the sampling period. Additionally, particular records were peremptorily excluded from the working database due to various factors that have been identified by the Stormwater Technical Team. These specific exclusions and associated uncertainty are discussed in Appendix E [Section 3.0](#).

The methodology for calculating stormwater loading assumes that concentrations measured in individual sampled outfalls at non-representative sites are indicative of concentrations for all stormwater discharging from the site. This methodology has inherent uncertainty associated with it, as concentrations can vary significantly based on the physical characteristics of the drainage basins associated with the stormwater discharges. For example, if a drainage basin that was sampled drains a known upland source area, the concentrations measured in this discharge ~~will~~ may be significantly higher than stormwater discharges at the remainder of the site. Thus, this example could overestimate stormwater loading for this site. Overall, however, the direction of any bias in the estimates created by these uncertainties is unknown.

Additionally, other more specific factors within this particular study’s methods that may contribute to the uncertainty of the stormwater loading estimates are discussed in Appendix E [Section 3.0](#).

#### 6.1.2.3 Summary of Findings

This section presents the findings of the stormwater loading analysis. Stormwater loading to the Study Area is presented and discussed for stormwater ~~IC~~ contaminants using both the stormwater composite water and sediment trap data sets, where available, as two independent ways of estimating loads.

In addition to Study Area stormwater loads, loading estimates by Fate and Transport Model cell are presented for PCBs, PAHs, and total DDx to provide insight into patterns of loading throughout the Study Area. Loading estimates for each ~~of the IC~~ selected contaminants and Fate and Transport Model cell for the entire Study Area are presented in both graphic and tabular format in Appendix E [Section 3.6](#).

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Table 6.1-4 presents the range of annual stormwater loads to the Study Area for each stormwater IC for both composite water and sediment trap data. These ranges are also presented graphically on Figures 6.1-21 through 6.1-35 for each chemical group, including ranges of the annual load estimated using both composite water and sediment trap data. The findings regarding patterns of total loads estimates are discussed for each chemical group in the following paragraphs. Refer to the maps in Appendix E [Section 3.6](#) (Maps E3-1a–b through E3-32a–b) for a graphical representation of loads for each ~~IC of the selected contaminants~~ throughout the Study Area. Tables 6.1-5a–b present a percentage comparison of loads to the Study Area by land use and non-representative location for both composited water and sediment trap data.

~~For total PCBs, the loads for total PCBs estimated using composited water data are slightly higher than the sediment trap estimated loads (Figure 6.1-21). The estimated loading rate for total PCBs is highest for the heavy industrial land use category as compared to other land uses, although one non-representative location contributes the highest estimated load. However, uncertainty in the calculated loading rates from non-representative sites due to sampling only one outfall from the site and then extrapolating these concentration results to stormwater runoff from the entire site may bias these results (see Section 6.1.2.2).~~ A comparison of loads of individual PCB congeners and total PCB TEQ are shown in Figures 6.1-22 and 6.1-23. Generally, composited water estimated loads for the various PCB components are slightly higher than the sediment trap estimated loads.

**Commented [KK27]:** New edit based on discussions between EPA and LWG on June 3 and 6.

Stormwater loads for DDx pesticides are presented in Figure 6.1-24. The results for these compounds show that the composited water estimated loads are generally within the range of loads calculated from the sediment trap data. The estimated annual loads for total ~~DDX-DDx~~ is highest for the heavy industrial land use category as compared to other land uses; however, the highest estimated loading rates are from non-representative sites in Basin 20 (RM 6.8 to 7.4W). ~~It should be noted that uncertainty exists in the calculated loading rates from non-representative sites because are based on the results from only one a single outfall was sampled (from the former DDT process area) that was included in summations of all land use types and these concentration results were then extrapolated to represent stormwater runoff from the entire site; therefore, by biasing the loading calculation. Thus, this estimate should be used with caution and may be biased high.~~ The estimated loads for non-DDx pesticides are highest for the heavy industrial land use category. Stormwater loads for non-DDx pesticides are presented on Figure 6.1-28. Generally, composited water estimated loads for non-DDx pesticides are typically higher than the sediment trap estimated loads.

**Commented [awc28]:** The suggested revised text explains the potential bias associated with this limited data set.

**Commented [KK29R28]:** OK as modified.

~~For total PAHs, annual load estimates for total PAHs using composited water data compared well with estimates using sediment trap data (Figure 6.1-25). The estimated load for total PAHs is highest for the heavy industrial land use as compared to other land uses, with four non-representative locations contributing a substantial portion to the total stormwater load. However, uncertainty in the calculated loading rates from non-representative sites due to sampling only one outfall from the site and then~~

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~~extrapolating these concentration results to stormwater runoff from the entire site may bias these results (see Section 6.1.2.2).~~

Commented [KK30]: New edit based on discussions between EPA and LWG on June 3 and 6.

Stormwater loads for BEHP are presented on Figure 6.1-26. BEHP annual loads estimated using composited<sup>14</sup> water data are higher than those generated using sediment trap data. The highest estimated loading rates for BEHP are from the heavy industrial land use areas.

Stormwater loads for hexachlorobenzene are presented on Figure 6.1-27. The results for hexachlorobenzene indicate that the sediment trap estimates are within the range of the composited<sup>14</sup> water estimates. The highest estimated annual loads for hexachlorobenzene are from the heavy industrial land use areas.

Stormwater loads for non-DDx pesticides (aldrin, dieldrin, gamma-HCH, and total chlordanes) are presented on Figure 6.1-28. For each of these chemicals, the composited<sup>14</sup> water annual load estimates were higher than estimates developed using sediment trap data.

Stormwater loads for metals are presented on Figure 6.1-29. Typically, for metals, the composited<sup>14</sup> water load estimates were slightly higher than estimates developed using sediment trap data. The highest overall estimated loads are observed for zinc, copper, and lead. Chromium, arsenic, and nickel have the next highest loads, and of the metals evaluated, mercury has the lowest. The highest estimated annual loads for metals are from the heavy industrial land use areas.

Stormwater loads to the Study Area are presented by river mile for total PCBs, total PAHs, and total DDx pesticides in Figures 6.1-30 through 6.1-35.

A graphical summary comparison of stormwater discharges to other loading terms is presented in Section 10.2 for the 13 CSM chemicals.

### 6.1.3 Permitted Point Source Discharges

Point source permitted non-stormwater discharges to the Study Area include NPDES-permitted discharges from commercial, industrial, private, and municipal outfalls or operations. This section presents the results of ~~semi-quantitative~~ estimation of the current annual mass load of chemicals from these outfalls to the Study Area. The details of data compilation and loading estimation are presented in Appendix E Section 4.0.

Both Oregon DEQ general and individual NPDES permits were considered in this evaluation. Active NPDES permits inside the Study Area were located using Oregon DEQ's Facility Profiler 2.0<sup>14</sup> and the DEQ Wastewater Permits Database<sup>15</sup> was used to

<sup>14</sup> Oregon DEQ's Facility Profiler 2.0: <http://deq12.deq.state.or.us/fp20/>

<sup>15</sup> DEQ Wastewater Permits Database: <http://www.deq.state.or.us/wq/sisdata/sisdata.asp>

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query the permit file numbers. As of this writing, there are 14 NPDES wastewater permitted discharges in the Study Area listed as either Individual or GEN 15A Permits. Map 6.1-1 shows the facility locations for these 14 permits. Note that this analysis is specifically limited to permitted wastewater discharges to the Study Area and does not represent stormwater discharges (included in stormwater loading term analysis; see Section 6.1.2) or other types of point sources.

Permitted direct discharge loading analyses were based on water chemical concentration data and discharge/flow data in discharge monitoring reports (DMRs), where available. These data were available for the following 10 of the 14 NPDES wastewater permitted discharges:

- EOSM
- Kinder Morgan/Portland Bulk Terminal 4
- Koppers Inc.
- Starlink Logistics, Inc.
- Siltronic Corporation
- ARCO Products Company
- Kinder Morgan Liquid Terminals
- Equilon Enterprises
- Pinnacle Condominium Complex
- Univar USA.

The remaining four NPDES wastewater permitted discharges listed below were not included in the loading calculations due to insufficient data for calculations and are shaded orange ~~in~~on Map 6.1-1:

The facilities and the reasons they were not included are:

- Ash Grove – No flow or chemical data reported
- Columbia River Sand and Gravel – No flow data reported and no chemical analysis required (only TSS and turbidity monitored)
- Vigor (Cascade General) – No flow data reported on DMRs
- Hoyt Street Properties – No flow or concentration data reported.

The discharge information from these sites would be expected to increase the upper and lower end estimates of total loading to the Study Area for the chemicals included in their permits. The lack of data for these facilities is not expected to represent a significant loading data gap for any parameters.

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Ranges of loading estimates were generated by considering the DMR discharge flow rates and chemical concentration data for all ~~IC-listed chemicals~~ the selected contaminants. Because of limited analyte lists in the DMRs and the permits, data for some IC-list parameters were not available for all facilities. Additionally, several ~~IC-list chemicals~~ of the selected contaminants were never monitored at any of the facilities. The results are summarized in Table 6.1-6 for the subset of ~~selected IC chemicals~~ selected contaminants for which data were available.

While there is uncertainty associated with the annual estimates for this loading term, the findings are expected to be reasonably representative of the relative significance of this pathway (as defined above) for current loading of ~~ICs~~ contaminants to the Study Area. The primary source of uncertainty in these estimates is the limited monitoring records available for many sites. ~~It should be noted that~~ there are four sites that could not be included in this assessment due to lack of information. If there is flow related to these permits, then discharge information from these sites would be expected to increase the upper and lower end estimates of total loading to the Study Area for the chemicals included in their permits. ~~It should also be noted that~~ this analysis is specifically limited to permitted wastewater discharges to the Study Area and does not represent stormwater discharges (included in stormwater loading term analysis; Section 6.1.2) or other types of point sources.

Review of these results indicates ~~first~~ that only a few of the analytes on the combined loading ~~IC-contaminant~~ list are presented in the DMRs (for one or more permit, results are presented for DDT, select PAHs, TPH, select metals, select VOCs, and cyanide). For all of the parameters analyzed, the estimated range of results is narrow—ranging over a factor of 5. While flow volumes are relatively large for some dischargers (total permitted discharge volume is estimated to be only slightly less than stormwater runoff), the concentrations ranges are low, and the resulting loads are generally low. Because of limited volume and low chemical concentrations, permitted point source discharges were not found to be a primary source of ~~ICs~~ contaminants to the Study Area for those facilities and parameters for which data was available. Overall, it is expected that this loading term, as defined and assessed here, is not currently a primary source of ~~ICs~~ contaminants to the Study Area since permitted discharges are regulated and monitored. A graphical summary comparison of permitted point source discharges to other external loading terms is presented in Section 10.2 for the 13 CSM chemicals.

#### 6.1.4 Atmospheric Deposition

Chemicals present in the atmosphere as a result of emissions from stationary sources (e.g., industrial smokestacks), mobile sources (e.g., vehicle emissions) and non-point sources (e.g., fugitive dust) produce a load to the Study Area through the processes of dry and wet deposition. Further, persistent chemicals can travel long distances through the atmosphere from other parts of the world. Dry deposition refers to the deposition of air pollutants from atmospheric suspension in the absence of precipitation. Wet

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deposition refers to deposition of air pollutants from atmospheric suspension via ~~liquid and/or frozen precipitation~~ rain or snow.

The following subsections present the approach and data sources applied to generate ~~semi-quantitative~~ estimates of the annual loading of selected analytes to the Study Area via dry and wet atmospheric deposition. Air deposition loading estimates presented here focus on dry and wet deposition directly onto the water surface of the ~~Lower~~ Willamette River within the Study Area. Atmospheric deposition to land in the Study Area watershed, which could subsequently be transported to the Study Area via stormwater runoff, is captured, though not distinguishable from other sources, in the stormwater loading assessment (Section 6.1.2). A qualitative discussion of atmospheric deposition to land in the Study Area watershed is provided in subsection 6.1.4.3.2.

The ~~IC-selected~~ contaminants list for atmospheric deposition loading is presented in Table 6.0-1. This list was limited to those chemicals on the combined IC loading list for which data were available to support the atmospheric loading estimates. The detailed data sets, methodologies, and results for dry and wet deposition loading to the Study Area water surface are presented in Appendix E Section 5.0. As with other loading term estimates, the atmospheric deposition estimates are presented as a range representing the relative uncertainty, as discussed further below.

#### 6.1.4.1 Data Sets and Approach

Atmospheric deposition is the sum of both dry and wet deposition loads. Under conditions of no precipitation, gases and particles are deposited to the ground or river surface in a process known as dry deposition. Dry deposition is driven by the gravitational force on the particulate matter and the gas aerosol. Numerous studies have been performed to characterize the dry deposition of various chemicals onto a variety of ground surfaces. EPA has conducted a review of many of these studies and concluded that dry depositions of both particulate matter and gases will contribute to the chemical concentrations in soils and surface water bodies (EPA 2005a). Wet deposition During precipitation events, occurs when gases and particles ~~can be~~ scavenged by rain droplets, ~~frozen precipitation elements~~ (freezing rain, ~~or~~ snow), or fog droplets ~~that and are ultimately~~ deposited to the surface. ~~This latter process is known as wet deposition.~~ The following subsections briefly describe the data sets and approach applied to assess dry and wet deposition loading to the Study Area; additional detail on the calculations and data sources is provided in Appendix E Section 5.0.

##### 6.1.4.1.1 Dry Deposition to the River Surface

Atmospheric dry deposition to the Study Area was estimated ~~semi-quantitatively~~ based on an assumed deposition velocity, Study Area-specific and non-local air concentration monitoring data, and the Study Area surface water extent. For a given analyte, dry deposition loading (kg/yr) to the Study Area can be calculated as the product of the air concentration (mass/volume), the deposition velocity (length/time), and the surface area of the Study Area (length<sup>2</sup>). The rate of chemical deposition to a surface (deposition velocity) is a function of atmospheric turbulence, properties of the chemical species,

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and the relative reactivity of the species with the receiving surface (Seinfeld and Pandis 1998). Where available, Study Area-specific or local ambient air concentration data were used. For those chemicals for which local sampling data were not available, concentration values from publicly available data sources, including DEQ and EPA, were used. The assumptions and data sources applied to determine these factors are presented in detail in Appendix E [Section 5.0](#). In summary, ~~for dry deposition, local information~~<sup>16</sup> was used in dry deposition calculations for all the metals, ~~BAP~~BaP, naphthalene, TPH (diesel), total PCBs, hexachlorobenzene, total PAHs, and total cPAHs; exclusively external data sources were used for dry deposition estimates for the rest of the atmospheric deposition ~~ICs~~contaminants. A range of estimates was generated for the dry deposition loading fraction of the total atmospheric load. This range was based on the range of ambient air concentration results compiled.<sup>17</sup> Specific effort was made to analyze the local monitoring and modeling data for ~~BAP~~BaP and naphthalene to ensure the representativeness of the data values for dry deposition loading over the river surface (see Appendix E for details).

#### 6.1.4.1.2 Wet Deposition to the River Surface

~~Wet deposition describes the process by which chemicals in particle and gas phases are scavenged from the atmosphere (in cloud and below cloud) by liquid or frozen precipitation and transported to the earth's surface. W~~Although wet deposition flux can be ~~modeling-modeled numerically; however,~~ the most reliable ~~estimation way to estimate wet deposition method~~ is to collect precipitation in suitable samplers, measure the chemical concentrations, and calculate the deposition flux corresponding to the sampling period (Reinfelder et al. 2004). Subsequently, the total annual wet deposition loading is calculated by multiplying the deposition flux by the total area of the Study Area. Unfortunately, such data are limited. From the ~~IC~~contaminant list, Study Area-specific wet deposition monitoring results were only found for total PCBs (MWH 2008) and mercury. In the MWH (2008) study, wet deposition data were collected from three monitoring stations within the Study Area for a two-month sampling period spanning May through June of 2007. The MWH (2008) study reported wet deposition loading rates calculated from the monitoring concentration data (taking into consideration the field blank values). ~~M~~For mercury, findings from Hope (2005) were considered for comparison with estimates based on the New Jersey Atmospheric Deposition Network (NJADN) data (Reinfelder et al. 2004; ~~see next paragraph~~). Briefly, the Hope ~~(2005)~~ study used precipitation monitoring data from Oregon Mercury Deposition Network sites (one site near Beaverton and one site near the southern end of the basin), and found wet deposition estimates comparable to those generated here by the NJADN ratio

<sup>16</sup> Local is defined here as monitoring data or modeling results for Portland, Oregon or Multnomah County, Oregon.

<sup>17</sup> ~~The~~For BaP, a maximum value of 0.32 µg/m<sup>3</sup> BaP was determined to be an outlying value among the values from the LASAR data based on statistical analysis and was excluded from the calculation; an average value of 0.19 µg/m<sup>3</sup> was also excluded for the same reason. ~~For naphthalene,~~ the following values ~~for naphthalene~~ were excluded from calculations based on statistical analysis: 2.16 µg/m<sup>3</sup> as one of the maximum values, 1.87 µg/m<sup>3</sup> as an average value, and 1.55 µg/m<sup>3</sup> as a minimum value. Please see Appendix E for more details.

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approach described in the following paragraph.<sup>18</sup> In summary, for wet deposition calculations, local information was used for mercury (Hope 2005) and total PCBs (MWH 2008), and non-local/modeling data sources were used for all other chemicals.

For the ICs other than With the exception of PCBs and mercury, for which lack Study Area-specific precipitation monitoring results were available, the monitoring results from NJADN (Reinfelder et al. 2004) were used, corrected by the ratios of 1) total atmospheric concentrations between Portland, Oregon and Jersey City, New Jersey (where available in both), and 2) total annual precipitation between Portland and Jersey City. This approach of scaling NJADN data sets to develop wet deposition loading estimates generated only one a single point estimate (not a rather than a range of estimates) for each IC of annual wet deposition loading to the Study Area because only the average values were reported from the NJADN study. The detailed methodology, data sources, loading results, and associated uncertainties are presented in Appendix E Section 5.0. Uncertainty and results are also discussed in Section 6.1.4.2 and 6.1.4.3, respectively.

Commented [awc31]: Suggest text revisions for clarity. Per the last sentence in the paragraph above, local data was used for PCBs and mercury; NJADN data for everything else.

Commented [KK32R31]: OK

#### 6.1.4.1.3 Total Deposition to the River Surface

For each IC, the total deposition loading to the Study Area for each selected contaminant was estimated simply by summing the dry deposition and wet deposition loading estimates. Since only central estimates could be generated for wet deposition loading, the ratio of the central estimate for wet deposition to the central estimate for dry deposition was assumed to be representative of the ratios across the range of wet deposition loading estimates. From this, upper and lower range estimates were generated for wet deposition for use in estimating the total deposition range. Where wet deposition data were inadequate to allow for estimation of even a central estimate, total loads were assigned based on the dry deposition estimates. Wet deposition estimates were unavailable for PCB TEQ, TCDD TEQ, 4,4'-DDE, 4,4'-DDT, naphthalene, total PAHs, DRH TPH (diesel), hexachlorobenzene, aldrin, and dieldrin. The estimates are still considered to be useful, however, based on the relatively low contribution of wet deposition to the total estimates for similar chemicals: total DDx (<2 percent), BAP BaP (~10 percent) and total cPAHs (~21 percent), and total chlordanes (~16 percent). This source of uncertainty is noted in the following subsection.

Commented [awc33]: As noted previously, retain "BAP" and "DRH" terminology to avoid needing to edit to other sections and data products. A change from BAP to BaP and from DRH to TPH (diesel) ripples through numerous figures and tables (Sec. 5, 6, and 10).

Commented [KK34R33]: This term is not used in any other section. The eco risk assessment uses TPH.

#### 6.1.4.2 Uncertainty

The lack of the Study Area-specific, analyte-specific, and temporally proximal data inputs for many of the ICs contaminants places significant uncertainty on the estimates for the atmospheric deposition loading term. Specifically, local data were available only for metals, BAP BaP, naphthalene, cPAHs (modeled), total PAHs (modeled; based

<sup>18</sup>Hope (2005) calculated dry, wet, and total mercury loading rates to surface water for the entire Willamette River basin (398,000,000 m<sup>2</sup>). When scaled down to the sub-area of the basin represented by the Study Area (8,791,735 m<sup>2</sup>, 2 percent of the open water area estimated by Hope), Hope estimates a total atmospheric mercury load of 0.08 kg/yr. This result is slight lower than, but comparable to, the lower mercury load (0.11 kg/yr) presented here.

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on 16 individual PAHs), hexachlorobenzene, ~~DRH~~TPH (diesel), and total PCBs (modeled) for dry deposition calculations; for wet deposition calculations, local data were available only for mercury and total PCBs (limited sampling period). In the case of the atmospheric deposition loading estimates, the presented range of estimates (lower, central, upper) is not expected to fully capture or represent the uncertainty associated with this term, due to significantly limited local empirical data. ~~The uncertainty varies by ICcontaminant, and is discussed qualitatively (noting data sources and findings relative to empirically estimated terms) in Section 10.~~

Commented [KK35]: New edit based on discussions between EPA and LWG on June 3 and 6.

The major uncertainties ~~For associated with~~ dry deposition loading estimates, ~~the major uncertainties~~ are as follows:

- The limited available local atmospheric concentration data
- The necessarily simplified calculation methodology
- The uncertainty associated with selection and uniform application of a deposition velocity.

The major uncertainties ~~For associated with~~ wet deposition loading estimates, ~~the major uncertainties~~ are as follows:

- The ~~extremely~~ limited local wet deposition monitoring data ~~Empirical data were available only for mercury and data only found for PCBs, [only a partial year of sampling record available] and mercury~~
- The uncertainty associated with application of precipitation correction factors to allow for use of NJADN data.

In summary, atmospheric deposition to the river surface is expected to be one of the most uncertain loading terms, primarily due to the limited availability of local atmospheric concentration and precipitation concentration monitoring data, ~~atmospheric deposition to the river surface is expected to be one of the most uncertain loading terms~~. The direction of any bias in the estimates created by these uncertainties is unknown. Fortunately, deposition to the watershed and subsequent runoff to the river is captured in the empirical stormwater runoff data set and stormwater loading estimates (discussed in Section 6.1.2).

### 6.1.4.3 Findings

This section presents the findings ~~from of~~ the ~~semi-quantitative~~ estimation of atmospheric deposition to the river surface described above. A qualitative discussion of atmospheric deposition to the watershed is also provided.

#### 6.1.4.3.1 Atmospheric Deposition to the River Surface

Table 6.1-7 presents the estimated ranges of annual total atmospheric deposition to the river surface for the entire Study Area. Figures 6.1-36 through 6.1-40 graphically

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present the estimated ranges of annual loads for dry deposition, wet deposition, and total atmospheric deposition to the Study Area for each ~~IC-contaminant chemical~~ group.

**PCBs and TCDD TEQ** – The estimated ranges of dry, wet, and total deposition for total PCBs and TCDD TEQ are presented ~~in on~~ Figure 6.1-36. The dry deposition fraction of the annual load represents the majority of the total annual loading estimate ~~For for~~ total PCBs, ~~the dry deposition fraction of the annual load represents the majority of the total annual loading estimate,~~ with only less than 0.5 percent of the load attributed to wet deposition. No wet deposition data were available for PCB TEQ or TCDD TEQ estimates.

**Pesticides** – The estimated ranges of dry, wet, and total deposition for pesticides are presented in Figure 6.1-37. The total annual loads for pesticides are dominated by the dry deposition load estimates; ~~however~~ However, it should be noted that wet deposition estimates were only available for DDx and total chlordanes. ~~Of these, wet deposition, and~~ composed 2 percent and 16 percent of the total, respectively (see Appendix E, Table E5.2). Further, ~~it should be noted that the~~ total DDx estimates based on NJADN estimates are lower than the 4,4'-DDE and 4,4'-DDT estimates based on ATSDR ambient concentration estimates. This difference reflects the range of uncertainty in the various data sources. In light of this uncertainty, total DDx estimates to be used in the CSM discussion in Section 10 will reflect the combined ranges of the ~~estimates~~ estimates (lower = 0.0068 kg/yr, central = 0.17 kg/yr, and upper = 0.21 kg/yr).

**PAHs** – The estimated ranges of dry, wet, and total deposition for PAH ~~ICs~~ are presented ~~in on~~ Figure 6.1-38. The total annual loading estimates are significantly higher for naphthalene than BaP-BaP (10 times for upper value, 3 times for central, and about the same for lower value), suggesting dominance of the LPAH fraction. Further, for all PAHs, the dry deposition fraction of the annual load represents the majority of the total annual loading estimate, with only a very small fraction ~~of the load~~ attributed to wet deposition.<sup>19</sup> (Total PAH atmospheric loads are based on 16 PAHs from Oregon EPA National Air Toxics Assessment data [EPA 1996], which includes all of the Study Area PAHs except for 2-methylnaphthalene.) The PAH loading estimates are considered to be highly uncertain based on comparison with other loading term estimates, ~~as discussed in Section 10.2.~~ Furthermore, statistical analysis EPA LASAR data for BaP BaP and naphthalene indicated some data values are out of the statistical ranges that are suitable for atmospheric loading calculations, therefore, the total PAHs values could be affected by the outliers.

Commented [KK36]: New edit to remove reference to Section 10.

<sup>19</sup> Wet deposition data were not available for total PAHs based on Oregon EPA NATA data (EPA 1996) for direct calculation of wet loading estimates; however, a closer look at the NJADN data set suggests that wet deposition is not expected to be a significant fraction of the total deposition for this chemical set. Wet deposition data were available from the NJADN study for a total based on 36 PAHs. Analysis of that New Jersey data shows that wet deposition loads are 3 orders of magnitude lower than dry deposition loads. Similarly, analysis of the 13 Study Area PAHs included in the New Jersey data set of 36 also shows that wet deposition loads are 3 orders of magnitude lower than dry deposition loads.

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~~Diesel-Range Hydrocarbons~~**Total Petroleum Hydrocarbons (diesel) and Hexachlorobenzene** – The estimated ranges of dry and total deposition for DRH and hexachlorobenzene are presented ~~in~~on Figure 6.1-39. No wet deposition data were available for these chemicals. Furthermore, as discussed in Appendix E, no data to support estimates of dry, wet, or total atmospheric deposition rates were available for other TPH fractions (i.e., gasoline-range and residual-range hydrocarbons).

**Metals** – The estimated ranges of dry, wet, and total deposition for metals are presented ~~in~~on Figure 6.1-40. Lead, zinc, and copper ~~showed~~exhibited the greatest total annual loading estimates by atmospheric deposition. ~~For the metal ICs assessed, the dry deposition loading contribution to total annual deposition was greater than the wet deposition contribution, with the exception of mercury, which exhibited seven times greater annual deposition by wet deposition. While dry deposition estimates were greater than wet deposition~~ For for the other metals, ~~while dry deposition estimates were greater than wet deposition, the dry deposition estimates were all within a factor of 10 of the wet deposition estimates, suggesting both mechanisms are important to the overall load.~~

~~Comparison between these estimated atmospheric annual loads to the surface water and loads estimated for other loading mechanisms is presented on a chemical by chemical basis in Section 10.2.~~

Commented [KK37]: New edit to remove reference to Section 10.

#### 6.1.4.3.2 Atmospheric Deposition to the Watershed

Chemicals that are deposited via atmospheric deposition to soils and impervious surfaces in the Study Area watershed may subsequently be transported to the Study Area via stormwater runoff. In general, for surface water bodies with relatively smaller watershed areas ~~(e.g., the Great Lakes)~~ compared to water surface area, the total atmospheric deposition loading to the surface water is greater than the deposition loading to the watershed (Steuer 1995). But for a riverine system such as the Lower Willamette River, with small surface water areas relative to the contributing watershed, atmospheric deposition to the watershed plays a greater role.

Commented [awc38]: Retain the reference to the Great Lakes because this example is illustrative of the point being made.

Commented [KK39R38]: Either delete entire sentence or delete reference to Great Lakes. It has no reference to the site.

~~The~~A review of available literature indicates that the relative importance of the atmospheric deposition loading term, relative to other loading terms, varies by site and by chemical. Some studies found atmospheric deposition to the watershed to be a significant source to the surface water bodies. For instance, atmospheric deposition was found to be the dominant source term for total PCBs to the North and Baltic Seas (Struyf and Van Grieken 1993; Wania et al. 2001) and for HCH to the North Sea (Struyf and Van Grieken 1993). A recent study performed by Sun et al. (2007) in the Great Lakes region correlates average gas-phase atmospheric PCB concentrations with local population size, suggesting a strong urban source of atmospheric PCBs. Likewise, Motelay et al. (2006) found atmospheric deposition to impervious surfaces to be the most important source of PAHs to the urbanized Seine River basin near Le Havre, France. Further, one of the most recent systematic monitoring studies (the NJADN) found that direct (dry, wet, and gaseous air-water exchange) and indirect (runoff)

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atmospheric deposition are of major importance to the accumulation of certain elements (e.g., mercury) and major nutrients in surface water ecosystems (Reinfelder et al. 2004). Findings from a separate, locally relevant study led by Hope (2005) of Oregon DEQ produced loading rate estimates for mercury comparable to those from the NJADN study.

Other studies found atmospheric deposition to the watershed to be less significant as a source of chemicals to surface water. A study of numerous urban U.S. streams (not including the Willamette River) evaluated the relative importance of different non-point sources of VOCs to total loading, finding that atmospheric deposition was of secondary importance for VOCs compared to the loading from urban land sources (Lopes and Bender 1998).

Because of the complexity of the fate and transport of chemicals via stormwater runoff, a simple application of the flux rate estimated for deposition to the water surface is not appropriate for estimating loads to the Study Area from atmospheric deposition to the watershed. Chemicals deposited in the watershed surfaces are subject to a number of loss mechanisms outside of runoff transport, including leaching, degradation (biotic and abiotic), and volatilization (EPA 2005a). Further, it is difficult to appropriately estimate the amount of deposited chemical mass that would be transported by runoff, and even more difficult to determine how much of that entrained chemical mass would be transported to the Study Area surface water given the complexity of routing and settling, etc. along the pathway. Other studies (Deletic et al. 1997; Grottker 1987) highlight the complexity of quantitatively estimating the relative contribution of atmospheric deposition to surface water bodies. These studies note that such estimates require a detailed understanding of the geochemical process and transport fluxes specific to the urban watersheds.

Keeping in mind the complexity noted above, the only empirical information available to assess the atmospheric contribution to the stormwater load is present in the stormwater data set. While many areas sampled as part of the LWG stormwater program have ~~IC~~contaminant sources other than atmospheric sources, it could be assumed that samples collected from open space areas (and possibly residential areas, depending on the ~~IC~~contaminant) represent primarily atmospheric deposition sources. TAA target ~~ICs~~contaminants for stormwater loading ~~were not~~were detected in stormwater runoff in water and/or sediment trap samples in all sampled open space land-use type locations, except for the contaminants 4,4' DDD, total DDD, aldrin, dieldrin, gamma-HCH, hexachlorobenzene, naphthalene, PCB 081, PCB 126, PCB 169, and total chlordanes. PCDD/Fs and TPH were not sampled in stormwater runoff for any land-use type. Given the complexities/variables of runoff routing, adsorption of chemicals to varying surfaces, stormwater controls, etc., a more rigorous assessment of the stormwater data set is not warranted, given the complexities/variables of runoff routing, adsorption of chemicals to varying surfaces, stormwater controls, etc. These variables confound the utility of a direct comparison of open-space runoff to other land-use type runoff for the purposes of assessing atmospheric deposition contributions. A

Commented [KK40]: Remove "the contaminants", the rest are OK. Changed ICs to contaminants.

Commented [awc41]: The modifications suggested here incorrectly changed the meaning of this sentence. The intent of the sentence is to state that these listed chemicals were not detected, all other chemicals were detected. The word "not" should be removed.

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discussion of stormwater data by land use type is presented in Section 4, and the stormwater annual load estimates by land use type were presented in Section 6.1.2.

### 6.1.5 Upland Groundwater Plumes

Upland groundwater plumes flowing toward the river are a ~~possible-potential~~ source of chemicals to the in-river sediments, TZW, and surface water in the Study Area. This section presents the approach, data sources, and findings of an ~~an~~ ~~quantitative~~ estimation of the loading of ~~chemicals-contaminants~~ to the Study Area from upland groundwater plumes. Empirical seepage rate and TZW concentration data information from the nine GWPA study sites (~~a detailed discussion of site selection and GWPA data interpretation see is presented in Appendix C Section 2~~) ~~for detailed discussion of site selection and GWPA data interpretation~~) were applied to generate an estimated range of annual loads for the individual study sites.<sup>20</sup> There may be additional sites that lack upland groundwater data but have complete groundwater pathways; however, such sites have not been identified or assessed.

The ~~chemicals-contaminants~~ listed in Table 6.0-1 ~~as upland groundwater plume loading ICs were are~~ the focus of this loading assessment. ~~This IC list was based on, which and includes the TZW contaminants discussed in Section 5 ICs designated for discussion in Section 5 regarding in-river distribution (with the exceptions of the localized source chemicals of TPH, cyanide, perchlorate, and Silvex), as well as chemicals contaminants to be assessed in the Fate and Transport Model under development for the FS and the RI CSM presented in Section 10.~~

~~It should be noted that,~~ in order to generate estimates for this loading term, a simplifying assumption was applied that is ~~recognized to not be~~ reflective of actual conditions in all areas/for all ~~ICs-contaminants~~. Specifically, these estimates assume that observed TZW concentrations are entirely attributable to upland groundwater. In areas where there are both upland groundwater plume and sediment sources ~~of chemicals, chemicals-contaminants~~ detected in TZW samples may be partly or wholly attributable to ~~chemicals-contamination~~ originating in sediment solids (partitioning into pore water). For certain ~~chemicals-contaminants~~ (e.g., redox-sensitive metals, petroleum-related hydrocarbons, etc.), as discussed in Appendix C ~~Section 2~~, differentiation of the origin of ~~chemicals-contaminants~~ present in the pore water in areas with groundwater discharge and upland groundwater plumes was often not possible with the available information. In such instances, the estimates of groundwater plume loading are expected to be redundant with advective loading estimates in the specific TZW study areas. ~~Advective loading estimates based on equilibrium partitioning~~

**Commented [KK42]:** As discussed on June 3, these are not the same contaminants that are now discussed in Section 5 so deleted this paragraph and first sentence is added to next paragraph.

**Commented [KK43]:** Delete this language – not relevant nor accurate. Include 1<sup>st</sup> sentence with next paragraph.

<sup>20</sup> ~~It is acknowledged that these~~ Study Area loading estimates are based on empirical information from only nine study sites, and it is possible that other sites will be identified that have a complete pathway for upland groundwater plumes to the Study Area. The groundwater pathway site selection process was designed to identify all sites with a reasonable likelihood of exhibiting a complete transport pathway for upland groundwater plumes to the Study Area. Detailed discussion of the groundwater pathway site selection process is presented Appendix C ~~Section 2~~.

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assumptions and sediment concentrations are presented in Section 6.1.6.) This redundancy was recognized and accepted in this analysis to allow for assessment/approximation of each of the terms and comparison of the relative, Study Area-wide effects. ~~(Note that loading estimates for the Fate-fate and Transport transport Model-model~~ will address loading from upland groundwater plumes and advective loading from groundwater discharge through sediments somewhat differently. The model simulates the transport of chemical within, and out of, the sediment bed via processes such as advection due to movement of groundwater, diffusion, and dispersion; this transport includes partitioning. In the specific areas where there are contributions from upland plumes, an upland plume loading term is specified based on available TZW concentrations and flux estimates from filtered trident and peeper data; this additional mass is subject to the same transport processes and partitioning within the bed.)

The following subsections present a summary of the data sets and approach used in the upland groundwater plume loading calculations, as well as a presentation and discussion of the findings. Detailed presentations of the data sets, data treatment, calculations, assumptions, and results are presented in the supporting Appendix E [Section 6.1](#).

#### 6.1.5.1 Data Sets and Approach

Estimates of groundwater plume ~~chemical-contaminant~~ loading to the Study Area are based on site-specific identification of potential plume discharge zones offshore of the nine TZW study sites, measured concentrations of ~~ICs-contaminants~~ in TZW, and measured groundwater discharge rates in potential plume discharge zones. The following data sources were used to determine these terms:

- Twenty-eight flow zone areas identified offshore of the nine TZW study sites were used to group data sets for the calculations. These flow zones are presented with discussions supporting the interpretations in Appendix C [Section 2](#). The zones are also presented in Appendix E in support of the detailed approach discussion presented in Appendix E [Section 6.1](#).
- Measured shallow TZW ~~chemical-contaminant~~ concentrations from 150 sample locations at the nine study sites were applied to the calculations. These samples represent the complete TZW data set for the sample depth interval from 0 to 38 cm bml (see Map 2.2-6). The sampling methods used to produce this data set include small-volume peeper, Trident, and Geoprobe samplers. Both unfiltered and filtered (where available) results were included in the evaluation. These TZW analytical results were presented in detail and discussed in Section 5.4.
- Seventy-seven seepage meter measurements from the 28 flow zone areas were used to estimate groundwater flux for each zone. This seepage rate data is presented in Appendix C [Section 2](#).

As a first step, Thiessen polygons<sup>24</sup> based on the TZW sampling locations were generated within each flow zone based on the TZW sampling locations to assign an area to each sample. This step was necessary to support area-weighting of each TZW analytical result. Loading estimates were prepared for each flow zone area by summing the estimated loads for each of the sample polygons within the flow zone, using the following general equation:

$$Load_{flowzone} = \sum(C_{sample} \times A_{sample} \times UnitFluxRate)$$

Where,

$Load_{flowzone}$  = the estimated annual mass load to surface water,  $\mu\text{g/yr}$

$C_{sample}$  = the chemical concentration in the TZW,  $\mu\text{g/L}$

$A_{sample}$  = the area of the Thiessen polygon associated with the given sample,  $\text{ft}^2$

$UnitFluxRate$  = groundwater seepage flux rate for the given flow zone,  $\text{L/ft}^2/\text{yr}$ .

A range of load estimates for each flow zone was determined by applying both the filtered and unfiltered concentrations to the calculations, ~~per agreement with EPA~~, as well as the average and the maximum measured seepage flux for the given flow zone. From the resulting four estimates, the highest and lowest values were assigned to designate the range. The estimate based on the average measured flux and the unfiltered concentrations was assigned as the central estimate.

The range of estimated annual loads for a given study site was determined by summing the estimated ranges for each Thiessen polygon. The ranges of load estimates for the Study Area were, in turn, generated by summing the estimates for each of the nine study sites. Detailed presentation of the steps in this approach and complete tabulated results are provided in Appendix E [Section 6.1](#).

#### 6.1.5.2 Uncertainty

The upland groundwater plume loading estimates are based on empirical, Study Area-specific TZW chemistry and groundwater flux data collected offshore from the nine upland sites included in the GWPA sampling program conducted as part of the RI. The range of results presented for this term is expected to be a reasonable approximation of the uncertainty in the loading estimates, though there are additional potential sources of uncertainty that may not be reflected in these ranges. Specifically, the following

<sup>24</sup> Thiessen polygons are formed as a network of polygons generated around seed points. In this case, the seed points are sampling locations. The polygon around each seed point delineates all areas that are closer to the seed point than to any other seed point.

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sources of uncertainty are acknowledged in the upland groundwater plume loading estimates:

- This assessment does not include loading from sites other than the nine study sites where empirical TZW data were collected. As described in the site selection process (~~see~~ Section 4.4.3.1 and Appendix C ~~Section 2~~), these nine sites represent those with a confirmed or reasonable likelihood for discharge of upland groundwater COIs to Portland Harbor. Eighty-three other upland sites reviewed during the site selection process lacked sufficient data to determine the completeness of the groundwater pathway. To the extent that a complete groundwater transport pathway to the Lower Willamette River could be identified in the future at one or more of these 83 sites or other currently unidentified sites, total groundwater plume loading to the Study Area may be underestimated.
- The spatial resolution of the analysis is limited to the resolution of the sampling data sets, as reflected in the Thiessen polygon approach.
- There is no attempt made in these estimates to distinguish the origin of the chemicals-contaminants in the TZW, and it is expected that the empirical TZW data set includes chemicals-contaminants originating from sediment contamination (as assessed in the advective loading analysis in Section 6.1.6 and Appendix E ~~Section 6.2~~).
- The GWPA study design specifically targeted areas of higher seepage and higher TZW concentrations for sampling in the areas offshore of the study sites.
- The TZW concentration estimates do not account for any additional chemical attenuation to sediments that may occur in the upper 38 cm bml.
- Sampling was conducted during the hydrologic season of highest expected groundwater flow rates to maximize the observed groundwater signal (plume concentration and flow rate). Consequently, the lower end of the groundwater signal in the discharge areas is not captured in the empirical data set.

### 6.1.5.3 Findings

The estimated ranges of upland groundwater plume annual loads are presented in Table 6.1-8 at the Study Area scale (sum of all nine study sites) and in Table 6.1-9 for the individual study sites. Groundwater plume loads at the Study Area scale are also presented graphically by chemical-contaminant group in Figures 6.1-41 through 6.1-45. These chemical-group plots show the estimated loads based on filtered and unfiltered estimates to allow for comparison of these data sets. Load estimates for the individual study sites are presented graphically in Figures 6.1-46 and 6.1-47 for total DDx and total PAHs only. ~~only. (Note: PCBs and dioxin/furans were not sampled in TZW.)~~

Figure 6.1-41 presents load estimates based on filtered and unfiltered TZW sampling data for ~~the DDx components on the upland groundwater plume loading IC list~~. The

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unfiltered results are consistently higher than the filtered results for this group of hydrophobic chemicals. As discussed in Section 5.4, unfiltered results are likely biased high due to entrainment of sediments in the TZW samples. Further, the DDD components compose the largest fraction of the total DDx load estimates for both filtered and unfiltered samples, while the DDE components make up the smallest fraction. Figure 6.1-46 presents the total DDx loading estimates for the two study sites where this ~~IC-contaminant~~ was sampled in TZW.

Figure 6.1-42 presents the upland groundwater plume loading estimates for ~~the PAHs on the IC list~~. The majority of the total PAH load from upland groundwater plumes is from LPAHs (~~e.g., naphthalene~~), as opposed to ~~the HPAHs (e.g., BAP and total cPAHs [cPAH BaPEq estimates follow total cPAH trends]; Figure 6.1-42)~~. This result follows behavior expectations that the LPAHs are more soluble than the HPAHs. Both HPAHs and LPAHs show a pattern of higher unfiltered concentrations and lower filtered concentrations. This pattern is expected of hydrophobic molecules such as PAHs.

Estimates for upland groundwater plume loading of metals at the Study Area scale (sum of all nine study sites) are presented on Figure 6.1-43. These estimates cover a large range of values, from a central estimate of approximately 0.02 kg/yr for mercury to a central estimate of 8,500 kg/yr for manganese. It is interesting to note that the unfiltered/filtered loading ratios vary for different metals. The ratios for ~~two metals, arsenic and manganese, show little difference; for barium, cadmium, nickel, and mercury unfiltered loading estimates are moderately larger-greater~~ than filtered estimates. ~~The remaining metals show large disparities between unfiltered and filtered loading estimates: Zn Zinc (31), Cu copper(86), and Pb lead exhibit large disparities between unfiltered and filtered loading estimates(120).~~ Estimated metals loads associated with groundwater discharges at individual study sites are provided in Table 6.1-9.

Figures 6.1-44 and 6.1-45 present the upland groundwater VOC and SVOC loading estimates at the Study Area scale (sum of all nine study sites). These plots are broken into two groups of VOCs: Group 1 includes ~~all of the~~ chlorinated, non-aromatic VOCs ~~on the IC loading list; Group 2 contains the aromatic VOCs, including BTEX chemicals, as well as 1,2-DCB and carbon disulfide.~~ Among the Group 1 VOCs (Figure 6.1-44), ~~it is apparent that~~ chloroform and methylene chloride dominate the loading scale. ~~cis-1,2-DCE. On the TCE degradation chain, cis-1,2-DCE showsexhibits the highest loads among TCE and its daughter products.~~<sup>22</sup> ~~Chloroethane exhibits the highest loads. On among the TCA degradation chain and its daughter products, chloroethane shows the highest loads.~~ Among Group 2 (Figure 6.1-45), benzene loads dominate the BTEX chemicals, and chlorobenzene loads are higher than 1,2-DCB. Estimated VOC and SVOC annual loading are provided in Table 6.1-9.

<sup>22</sup> ~~No~~ ~~te that the~~ loading estimates for TCE, cis-1,2-DCE, and vinyl chloride are dominated by results from a single TZW sample offshore of the Siltronic site. The groundwater pathway for TCE is discussed in detail in Appendix C ~~Section 2~~.

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Upland groundwater loading estimates are compared to other loading rate estimates as part of the CSM presentation in Section 10.

#### 6.1.6 Advective Loading

Advection of groundwater through contaminated sediments can transport chemicals that desorb from sediment solids into the aqueous phase and then migrate with the flowing groundwater. ~~a process defined as in this document, this mechanism is termed~~ "advective loading."<sup>2</sup> Two types of advective loading were evaluated for this RI Report:

- Subsurface advective loading: ~~is~~ migration of ~~chemicals contaminants~~ associated with subsurface sediments (deeper than 30 cm bml) to surface sediments via desorption and groundwater advection and sorption to surface sediments. ~~This is considered to be an external loading term to the Study Area per verbal agreement with EPA.~~
- Surface advective loading: ~~the~~ ~~is~~ migration of chemicals associated with surface sediment (0 to 30 cm bml) to surface water via desorption and groundwater advection. ~~This is considered to be internal fate and transport process Per agreement with EPA, surface advective loading to surface water is categorized in this RI as an internal fate and transport process (as opposed to an external loading term), as it. Surface advective loading thus involves chemical mass transfer between media within the Study Area (from surface sediment to surface water).~~

Section 6.1 generally focuses on external loading mechanisms, such as subsurface advective loading, which transport mass into the Study Area. However, ~~although considered strictly internal fate and transport process, surface advective loading, which is an internal fate and transport process, is also described in this section. The description is included here (as opposed to presentation of the surface advection term in the fate and transport discussion under Section 6.2) to allow for parallel description of the common calculation approach and for comparison of results for both surface and subsurface sediment advective loading. This section describes the approach and results of the analyses to generate estimates of subsurface and surface advective loads.~~

The surface and subsurface advective loading terms were assessed for the advective loading IC-list presented in Table 6.0-1. These ~~chemicals contaminants~~ were selected ~~from the complete list of loading ICs~~ because they are likely to sorb to sediment solids and are subject to the chemical partitioning processes relevant to this loading mechanism. Both terms were assessed on the scale of the relevant available sediment data set. Loading estimates for each term were generated in units of mass loading per year and presented for the entire Study Area and by river mile.

These advective loading estimates differ from the upland groundwater plume loading estimates ~~(presented previously in Section 6.1.5)~~ in that in-river sediment contamination, as opposed to upland groundwater contamination, is considered to be the

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chemical source for the advective loading analysis. In areas where ~~ICs concentrations~~ in pore water are attributable to both upland groundwater plumes and in-river sediment sources, the plume loading and advective loading assessments may overlap, resulting in some double-counting of loads. The extent of this overlap depends on the relative magnitude of the groundwater plume concentrations versus the sediment-derived pore water concentrations based on equilibrium partitioning. ~~(Note: the TZW plume study areas account for less than 5 percent of the Study Area) plan view area)~~ As described ~~previously~~ in Section 6.1.5, loading estimates for the ~~Fate-fate~~ and ~~Transport-transport~~ mModel address loading from upland groundwater plumes and advective loading from groundwater discharge through sediments somewhat differently than the empirical estimates developed for the RI.

### 6.1.6.1 Data Sets and Approach

The following subsections briefly describe the data sets and approaches applied to generate estimates of surface and subsurface advective annual loads, as well as accumulation rates of ~~chemicals-contaminants~~ in surface sediment. Detailed presentation of data sources and approaches is provided in Appendix E ~~Section~~ 6.2.

#### 6.1.6.1.1 Loading Estimates

The first step in the analysis ~~to generate annual loading estimates for surface and subsurface advective loading~~ was to ~~produce~~ ~~derive~~ Thiessen polygon sets for each ~~IC~~ ~~contaminant~~ for surface and subsurface sediment. ~~Each polygon represents Uniform sediment~~ ~~sediment~~ concentration, for the given IC, as well as a ~~sediment~~ bulk density, and a ~~sediment~~ organic carbon ~~value~~ are assigned to each polygon.

~~For Within~~ ~~For each polygon, a~~ Advective loading rates were developed ~~for each polygon~~ by first estimating pore water concentrations under an assumption of equilibrium, then applying an estimated advection rate to generate a mass annual load. This approach required two major assumptions. First, ~~it is assumed that the chemical partitioning between sediment and pore water is at a condition of~~ ~~assumed to be at~~ equilibrium at all times. Second, ~~it is assumed that~~ the groundwater advection rate is ~~assumed to be~~ constant throughout the system. ~~While these assumptions it is recognized that these assumed conditions~~ do not directly reflect the heterogeneity of conditions throughout the Study Area (limited assessment of proximal pairs of pore water and sediment concentration data are provided in Sections 6.2.1.1.1 ~~for~~ ~~for~~ organic ~~chemicals~~ ~~contaminants~~ and 6.2.1.1.2 ~~for~~ ~~for~~ inorganic chemicals), ~~however~~ However, both of these assumptions are considered necessary to allow for development of ~~semi-quantitative~~ loading estimates for the entire Study Area.

Pore water concentrations in ~~the surface sediment~~ and subsurface sediment were estimated for each sediment sample, ~~where surface sediment was designated as the data set representative of the top 30 cm bml, and subsurface sediment was defined as the interval descriptive of the concentrations immediately below 30 cm bml (detailed description of the data set definitions are presented in Appendix E6.2).~~ For each sediment sample, pore water concentrations were calculated for a range of equilibrium

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Commented [awc44]: Retain the word "sediment" for clarity.

Commented [KK45R44]: Leave in first occurrence, delete next two.

Commented [KK46]: OK as revised.

Commented [awc47]: Retain deleted text. Surface and subsurface are defined above, but re-defining here adds clarity. Also, the reference to App E for more details is important.

Commented [KK48R47]: No. Further, last sentence refers to Appendix E. Section 6.2.



partitioning values ( $K_{oc}$  or  $K_d$ ), applying the sediment ~~sediment~~ ~~sample-specific~~ percent solids value and organic carbon content (for non-metals). The formulas ~~applied~~ used and the steps to compile ranges of  $K_{oc}$  and  $K_d$  values, as well as the full compilation, are presented in Appendix E Section 6.2.

**Commented [awc49]:** Retain the word "sediment" for clarity.

**Commented [KK50R49]:** The beginning of the sentence already states that it is for each sediment sample. Either leave it the way EPA edited or sentence needs to read "Pore water concentrations were ...applying the sediment sample percent solids..."

The groundwater discharge rate was estimated using upland hydrogeologic data gathered ~~for under the Round 2 Report (Integral et al. 2007)~~, applying Darcy's Law to generate an estimated total discharge rate to the river, and converting that discharge to a unit flux through the river sediment surface. The complete data sets and the calculations are presented in detail in Appendix E Section 6.2. This approach generated estimates of total groundwater discharge rates to the Study Area of 4.6 cfs (lower estimate) to 11 cfs (upper estimate), with a central estimate of 7.3 cfs. Dividing by the surface area of sediments in the Study Area, this corresponds to a range of unit groundwater flux rates of 1.5 to 3.6 ft/yr, with a central estimate of 2.4 ft/yr.

**Commented [KK51]:** New edits based on discussions between EPA and LWG on June 3 and 6. The purpose of the sampling was not for the report.

The estimated groundwater discharge rate used in the advective loading calculations was compared with the flow rates observed by seepage meter measurements as part of the groundwater plume loading estimates. The Darcy's Law estimate range is roughly 15 percent of the unit discharge rate observed with seepage meters in the nearshore groundwater plume discharge areas. The selective placement of these seepage meters<sup>23</sup> and the measurement exclusively during hydrogeologic times of higher groundwater discharge (by sampling design) explains the disparity between the two ranges. Overall, the order-of-magnitude agreement between the unit flux rates developed using these two information sources offers confidence in the utility of the Darcy's Law-estimated rates.

From the ranges of estimated pore water concentrations, the range of groundwater advection rates, and the areas associated with each sediment sample,<sup>24</sup> ranges of advective loads were estimated for each sediment polygon by the following equation:

$$Load_{polygon} = C_{TZW} \times A_{polygon} \times Q$$

Where,

$C_{TZW}$  = the estimated chemical concentration in the pore water

$A_{polygon}$  = the area of the polygon

$Q$  = the estimated annual groundwater flux rate.

<sup>23</sup> In the design of the TZW study, seepage meters were purposefully placed at locations where there was an indication (based on pore water temperature measurements, sediment texture, or screening results) of higher flow rates. As such, the seepage meter measurements are expected to be biased high relative to an average unit discharge for the entire Study Area.

<sup>24</sup> Each sediment sample was assigned an area, based on Thiessen polygon sets generated for the surface and subsurface sediment data sets. This step is described in detail in Appendix E Section 6.2.1.

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Annual loading estimates for surface advection to surface water were arrived at by summing the loading assessed for all surface sediment polygons in the Study Area (or within a river mile). ~~For~~ subsurface advective loading (to the surface sediment interval), estimates were only generated for areas where OC-normalized subsurface sediment concentrations for the given analyte were greater than the corresponding OC-normalized concentration for the surface sediment.<sup>25</sup> This approach is described in greater detail in Appendix E, including graphical presentation of the spatial breakdown of this approach for PCBs, PAHs, DDx, and dioxin/furans, and a table of the spatial breakdown for all other advective loading analytes. In general, the percent of the Study Area with subsurface loading assessed to surface sediment ranges widely, from 6 percent for aldrin to 94 percent for total PAHs (these percentages reflect the percent areas where OC-normalized subsurface sediment concentrations exceed OC-normalized surface sediment concentrations, and are discussed further in the historical loading section below, Section 6.1.8). Table E6-4 in Appendix E summarizes the percent of the Study Area over which subsurface advective loading was assessed for the entire suite of chemicals assessed.

Mass loading estimates to surface sediment (by subsurface advective loading) and to surface water (by surface advective loading) were generated for the advective loading ~~HCs~~ contaminants following the approach described above. The range in each estimate reflects the range of equilibrium partitioning values and ~~the central~~ groundwater discharge rates ~~applied in~~ to the calculations. Specifically, the upper and lower advective mass loading estimates were calculated with the maximum and minimum equilibrium partitioning values, respectively. The primary equilibrium partitioning values (described in Appendix E, Section E6.2.2.2.1) were used to calculate the central advection loading estimate. These results are presented and discussed below in Section 6.1.6.3.

Commented [KK52]: Edit based on June 6 discussion.

#### 6.1.6.2 Uncertainty

There is significant uncertainty associated with the advective annual load estimates related to applied assumptions (~~including i.e.,~~ equilibrium behavior of all ~~HC~~ contaminants and uniform groundwater discharge rates), as well as the data sets used in the calculations (~~i.e.,~~ literature equilibrium partitioning coefficients, and roughly estimated groundwater discharge rates).

~~Related to equilibrium,~~ the primary uncertainty related to equilibrium is the assumption of equilibrium in all parts of the complex sediment/pore water environment at all times. This calculation fails to capture reaction kinetics and the sorption-

<sup>25</sup> Upward flux of chemicals from subsurface sediments due to desorption and advection can increase chemical concentrations in surface sediments if surface bulk sediment concentrations (OC-normalized) are initially lower than the underlying subsurface interval. If surface concentrations are greater than or equal to subsurface concentrations, mass flux from the subsurface will not accumulate in surface sediments but will instead discharge to overlying surface water. ~~It is assumed that the~~ existing estimates of advective flux from surface sediments to surface water are assumed to be inclusive of the latter phenomenon. Therefore, the scope of this analysis is restricted to locations in the Study Area where "cleaner over dirtier" conditions exist.

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desorption-resorption dynamics that occur in advective transport through sediment. For example, to the extent that non-equilibrium conditions may exist in the pore water environment as a result of kinetic limitations on desorption from contaminated sediments, the assumption of equilibrium will overstate pore water concentrations and advective loading rates. Beyond the assumption of equilibrium, ~~it should be noted that~~ the Study Area organic carbon associated with sediments may differ in character from that defined by the range of literature  $K_{oc}$  values. Likewise, the location-specific chemical and geochemical conditions (redox, pH, ionic strength and composition, sediment matrix composition, etc.) likely differ in character from those associated with the applied specific literature values. Further, this assessment ignores any chemical or biological transformation processes that may occur in the migration process.

~~There are a number of significant uncertainties. Related-related~~ to the groundwater flux rate estimates, ~~there are a number of significant uncertainties.~~ First, they are based on the limited available upland data and not on groundwater modeling of the area or direct measurement of seepage rates representative of the entire Study Area. Second, the groundwater advection rate estimates rely on a simple and conservatively high cross-sectional area. Third, the advection rate estimates apply a projection of the sediment surface area to represent the actual sediment surface area (thereby increasing the unit discharge estimate). Finally, the assumption of a uniform groundwater discharge rate for the entire Study Area does not capture the spatial variability that is inherent in groundwater discharges to rivers. The discharge rates are assumed to be constant in time and do not account for variability caused by seasonal recharge patterns, changes in river flow rates and stages, and tidal fluctuations (~~aka~~, “tidal pumping”). Among all seepage meter locations where net positive average advective groundwater fluxes were measured~~As discussed in Section 5.4.1.3, and presented in more detail in Appendix C2,~~ the largest net negative recharge rate measured in a seepage meter during a rising tide; ~~among all seepage meter locations where net positive average advective groundwater fluxes were measured,~~ was offshore of the Siltronic site (Appendix C Section 2). At this location, the negative recharge period covered roughly 9.5 hours, with an average seepage rate of  $-6.7$  cm/day. This corresponds to a net negative seepage flux of  $2.65$  cm into the sediment bed over the 9.5 hour tidal recharge period. Assuming sediment porosity of 25 percent, the maximum depth of influence for this period of negative seepage would be ~~approximately roughly~~  $10.6$  cm before the direction reversed to positive discharge with the tidal change. Although tidal pumping may in some instances lead to increased loading of chemicals from the sediment bed to the water column by introducing relatively clean surface water into the uppermost several centimeters of the sediment bed with each tidal cycle, uncertainty in the loading estimates due to this effect is expected to be minor compared to the other sources of uncertainty inherent in these calculations.

The large range in most of the estimates (presented in the following subsection) reflects the range in literature equilibrium partitioning coefficients. These large-scale estimates of advective annual loads are considered to be highly uncertain, but useful for general comparison to other loading terms for each chemical (~~Section 10~~). Any local

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recontamination concerns identified by the FS may require consideration of specific chemical and localized physical conditions, including ~~focused~~ collection of additional information and/or more complex assessments of advective loading.

### 6.1.6.3 Findings

This section discusses the results of the ~~calculations described above~~. This analysis ~~and~~ considers the results for surface sediment and subsurface sediment advective loading estimation for the entire Study Area for all advective loading ~~ICs~~ contaminants, as well as results by river mile for PCBs, PCDD/Fs, DDx, and PAHs.

#### 6.1.6.3.1 Study Area Annual Loading Estimates

Study Area-wide loading estimates for the subsurface ~~sediment~~ and surface sediment advective loading terms are presented in Table 6.1-10 ~~for all advective loading ICs~~. These results are also presented graphically on Figures 6.1-48 through 6.1-60, showing both surface and subsurface annual loading estimate ranges. Patterns and other observations for each of the ~~IC chemical~~ contaminants groups are discussed in the following paragraphs.

The central estimated ranges of annual loads for total PCBs (Figure 6.1-48) are slightly higher ~~from for the~~ surface sediment ~~to the surface water as compared relative to than for those from the~~ subsurface sediment ~~to the surface sediment~~. ~~Note~~, however, the difference for total PCBs (subsurface sediment advection versus surface sediment advection) is only a factor of ~~approximately~~ 1.5 for the central estimates. This observation ~~of slightly greater upper loading estimate for subsurface advective loading to surface sediments~~ is expected due to the Study Area-wide higher average PCB concentrations in subsurface ~~sediments~~ as compared to surface sediments. For the individual congeners, estimated load ranges were generally higher for subsurface sediment loading to surface sediments ~~as when~~ compared to advective loading from surface sediment advective loading to surface water (Figure 6.1-49). Of the individual congeners analyzed, PCB 118 and PCB 105 exhibit the highest annual loads, whereas PCB 169 is the smallest contributor.

Advective loading estimates ~~For for~~ PCDD/Fs (Figure 6.1-50), ~~the advective loading estimates~~ show a slightly greater loading from surface sediment advective loading to surface water, ~~as~~ compared to subsurface ~~advective loading~~ to surface sediments. Estimates of advective loading from surface sediment advective loading to surface water are higher by a factor of 2.5 for PCDD/Fs ~~as~~ compared to rates of subsurface ~~sediment advection partitioning~~ to surface sediments. The OC-normalized PCDD/F concentrations used in the load calculations are generally similar in surface sediment and subsurface sediment, with 54 percent of the subsurface ~~concentrations~~ ~~sediments~~ ~~showing being~~ greater ~~concentrations~~ than concentrations in the surface sediments (Table E6-4). ~~However, The the~~ Study Area-wide loading estimates, ~~however~~, are dominated by individual high surface sediment concentration values, resulting in the greater Study Area-wide total PCDD/F advective loading from surface sediment as compared to subsurface sediment.

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Commented [KK54R53]: OK

For DDX pesticides (Figure 6.1-51), DDD isomers ~~compose~~ comprise the largest share of the central estimate total DDX ~~advection load estimates load~~ for both surface sediment and subsurface sediment ~~advection load estimates~~, followed by DDT, and then DDE (Figure 6.1-51). The Study Area-wide subsurface loading ~~(to the surface sediment interval)~~ is slightly greater than the ~~advective~~ loading to surface water for each of the DDX pesticides compounds ~~evaluated~~ (by a factor of 1.8 for total DDX). ~~For DDXA~~, approximately 35 percent of the Study Area is not expected to exhibit advective loading of DDX from the subsurface ~~sediment~~ to the surface sediment due to higher OC-normalized sediment concentrations in the surface.

~~T~~For PAHs (Figure 6.1-52), total PAH annual load ~~is~~ from both the surface and subsurface sediments are dominated by the LPAHs (e.g., naphthalene) (Figure 6.1-52). The HPAHs (e.g., BAP and ePAHs [~~ePAH BaPEq estimates show similar trends to total ePAHs~~]) ~~show exhibit~~ slightly higher surface sediment loading to surface water as ~~compared relative~~ to subsurface sediment loading to surface sediments. In contrast, the naphthalene and total PAHs ~~loading show higher from~~ subsurface ~~sediment loading to~~ surface sediment ~~is greater than to surface water~~. ~~The average c with the estimated total PAH estimated subsurface advective loading being higher than the from subsurface to surface sediment is advective greater than loading to surface water by a factor of almost 3 for the central estimate. OC-normalized subsurface sediment concentrations of Note also that the LPAHs and the HPAHs on the IC list all exhibit are higher OC normalized subsurface sediment concentrations over 60 to 65 percent of the Study Area (Table E6-4).~~

Commented [awc55]: Retain "for the central estimate" for clarity.

Commented [KK56R55]: OK.

~~The range of advective load estimates for BEHP are presented on Figure 6.1-53 presents the range of advective load estimates for BEHP. The large 5 orders of magnitude range in the estimated loads — 5 orders of magnitude — for BEHP is a direct reflection of the large range in the literature  $K_{ow}$  (octanol-water partitioning coefficient) values. BEHP These estimates show slightly more surface sediment loading (by a factor of approximately 2 for the central estimates).~~

Commented [KK57]: Just missing capital of first word.

Commented [awc58]: EPA revisions seem to be missing text here.

Non-DDX pesticide advective loading estimates are presented on Figure 6.1-54. Among these, gamma-HCH ~~shows exhibits~~ the highest mass loading, ~~and while~~ aldrin ~~shows~~ the ~~lowest least~~ (possibly due to a tendency for aldrin to degrade to dieldrin in environmental systems). ~~T~~For the subsurface to surface advective loading ratios are 0.9 and 1.15 for aldrin and dieldrin, ~~the subsurface to surface advective loading ratio is~~ close to 1, at 0.9 and 1.15, respectively. ~~The This~~ ratio is lower for gamma-HCH (0.6) and higher for total chlordanes (~2.5). ~~N~~ ~~all of these non-DDX pesticides show exhibit~~ lower OC-normalized subsurface sediment concentrations ~~(as compared relative~~ to surface sediment) in over 85 percent of the Study Area, except ~~for~~ total chlordanes at approximately 60 percent (Table E6-4).

~~A~~The advective loading rate estimates for arsenic, copper, lead, and mercury are presented on Figure 6.1-55. Arsenic and copper show the highest Study Area-wide loading, followed by lead. Mercury ~~produced exhibits~~ the lowest loading estimates,

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with central estimates 4 orders of magnitude below the corresponding central estimates for arsenic and copper. ~~C~~For copper, mercury, and lead, the surface and subsurface loadings were ~~very close~~ essentially equal (ratios of 0.8 to 1.1), in agreement and representative with of the fairly even distribution of areas with higher or lower of surface and subsurface sediment concentrations<sup>26</sup> over (approximately 60 to 70 percent of the Study Area; (Table E6-4). ~~T~~For arsenic, the loading estimate for the arsenic surface sediment ~~advection~~ to surface water loading estimate is 2.2 times that for greater than the subsurface sediment loading to surface sediment estimated loading. ~~This is also reflected in the fact that and~~ only 40 percent of the Study Area exhibits subsurface concentrations greater than the in surface sediment concentrations (Table E6-4).

Commented [awc59]: The central estimates are not simple averages. Retain "central", do not replace with "mean".

Commented [KK60R59]: OK

Estimated TBT advective loads (Figure 6.1-56) vary over 3 orders of magnitude from the lower to upper estimates. The subsurface-to-surface sediment TBT loading estimate is 3.7 times higher than the surface sediment loading to surface water for the central estimates. ~~A~~For TBT, approximately 40 percent of the Study Area was not considered to be loading to the surface sediment, due to higher OC-normalized surface to subsurface sediment concentrations as compared to the subsurface (Table E6-4).

Commented [awc61]: The central estimates are not simple averages. Retain "central", do not replace with "mean".

Commented [KK62R61]: OK

In summary, Study Area-wide advective annual loads from subsurface sediment to surface sediment were higher than advective loading from surface sediment to surface water for PCBs, DDx pesticides, LPAHs (and total PAHs, which are dominated by LPAHs), BEHP, arsenic, total chlordanes, and TBT. The opposite was true only for PCDD/Fs, gamma-HCH, and HPAHs. There was little difference in-between the surface and subsurface advective loading estimates for aldrin, dieldrin, copper, mercury, and lead. These differences are a direct reflection of the patterns of relative OC-normalized concentration of each chemical in the surface sediment as compared to the subsurface sediment. This pattern is addressed further in the historical loading discussion (Section 6.1.8).

#### 6.1.6.3.2 Annual Loading by River Mile

Figures 6.1-57 through 6.1-60 present annual surface sediment and subsurface sediment advective loading for each river mile in the Study Area for total PCBs, total PCDD/Fs, total DDx, and total PAHs. Given the nature of the analysis (Thiessen-polygon-based scale of assessment), variations in river-mile scale annual load estimates are indicative of variations in sediment chemical concentrations and organic carbon content. Groundwater flux rates and assumptions of equilibrium behavior were held constant over the entire Study Area in advective loading calculations.

~~The sediment advective loading pattern for total PCBs is fairly complex. For total PCBs, the sediment advective loading pattern is fairly complex. As shown on Figure 6.1-57, shows that the highest central estimate advective loading from subsurface sediment to surface sediment for total PCBs are observed at RM 8 to 9, followed by the reaches at~~

<sup>26</sup> Note that for metals, the surface sediment and subsurface sediment concentrations comparisons to support the calculations for metals were not OC-normalized, reflecting the use of  $K_d$  values as opposed to  $K_{oc}$  values.

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the upstream and downstream ends of the Study Area. The highest annual surface sediment loading estimates to surface water are observed at RM 9 to 9.9, followed by the reaches at the upstream and downstream ends of the Study Area.

Subsurface sediment advective annual loading to surface sediment, and surface sediment advection to surface water For for total PCDD/Fs (Figure 6.1-58), both subsurface sediment advective annual loading to surface sediment and surface sediment advection to surface water are fairly consistent across the Study Area, with the exception of higher annual loading estimates to surface water from RM 7 to 7.9. No clear pattern of higher subsurface advective loading to surface sediments, as compared to surface advective loading to surface water, are apparent through the Study Area.

For DDx (Figure 6.1-59), the highest surface and subsurface sediment advective loads for DDx are predicted at RM 7 to 7.9 (Figure 6.1-59). Further, the DDx subsurface-to-surface sediment advective loading annual estimates are comparable or higher greater or comparable to than the estimates for of surface sediment advection to surface water in all river miles except RM 9 to 9.9 and RM 11 to 12.1.9, where the load from advection to surface water is somewhat greater.

The highest subsurface advective loading to surface sediment Finally, for PAHs (Figure 6.1-60), the highest subsurface advective loading to surface sediment is observed at RM 6 to 6.9.<sup>27</sup> The maximum surface sediment advection to surface water load estimates are observed at RM 5 to 5.9. For PAHs, the subsurface-to-surface sediment advective loading estimates are comparable to or higher greater or comparable to than the load estimates for surface sediment advection to surface water in all river miles except RM 5 to 5.9 and RM 9 to 9.9, where the load from advection to surface water is somewhat greater. (where they are what?)

### 6.1.7 Riverbank Erosion

This section assesses the susceptibility of Study Area riverbanks within the Study Area to erosion, and presents the limited data set available for chemically characterizing riverbank materials. No quantitative estimation of the loading of chemicals contaminants to the in-water portion of the Study Area from bank erosion is performed in this section, per agreement with EPA presented.

For the purposes of this analysis, riverbank materials are defined as soil and sediment that are between the mean high water mark (MHWM)<sup>28</sup> and the ordinary high water

Commented [A63]: Does this mean surface sediment loadings to water are greater here?

Commented [awc64]: Added text to clarify that the surface sediment to water advection is slightly higher than the subsurface to surface sediment advection for MR 9 – 9.9 and 11 – 11.9.

Commented [KK65R64]: OK

Commented [A66]: Provide what the load estimates are for this exception.

Commented [awc67]: Added text to clarify that the surface sediment to water advection is slightly higher than the subsurface to surface sediment advection for MR 5 – 5.9 and 9 – 9.9.

Commented [KK68R67]: OK

Commented [awc69]: Note that the number is deleted from footnote 27 below. The intent of this edit is not clear. The footnote should be retained because it includes important information defining MHWM.

Commented [KK70R69]: Footnote should be retained.

<sup>27</sup> This peak comprises the majority of the Study Area total load assessed for this term, and is attributed largely to LPAHs (see Table E6-7 and E6-8 in Appendix E).

<sup>28</sup> The MHWM is the elevation defining the shoreline boundary of the Portland Harbor Superfund Site, which is +13.3 ft (NAVD88). This elevation is based on a DEQ memorandum dated July 9, 2003 to EPA regarding the upland/in-water boundary for the Superfund Site (DEQ 2003b).

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mark (OHWM).<sup>29</sup> This includes gently sloped upper beach areas as well as steeper bank areas. ~~While e~~Erosion of bank materials containing ~~chemicals-contaminants~~ from above the OHWM, including materials much farther upland, is also possible; ~~however, such~~ upland erosion is primarily caused by overland transport (stormwater runoff), which is discussed in Section 6.1.2, and ~~by~~ flood events.

The primary mechanisms for riverbank erosion are river water moving over bank materials, direct overland transport across these materials, and sporadic mass wasting or slumping events when bank slopes become over-steepened or otherwise unstable. Wind erosion, shoreline construction and other human activities, activities of animals, ~~etc.~~ are also possible erosion mechanisms; ~~however~~ However, these can reasonably be considered to be minor in comparison to river and stormwater flows. ~~(Note: construction is considered minor because such projects are regulated and permitted to minimize erosion of bank materials into surface waters.)~~

River water can cause erosion ~~at times~~ when river levels rise and come into contact with the bank. The MHW (elevation +13.3 ft NAVD88) is based on the monthly average water level for the 16-year period from 1987 to 2002. ~~Thus, during some certain~~ periods, particularly during winter months, riverbanks above this elevation can become inundated by river water. Erosive mechanisms during these periods include the direct and shear stress forces of currents with sufficient nearshore velocity to suspend soil and sediment particles. Nearshore velocities can be affected by a number of factors, including the following:

- Bends in the river, where outer bends tend to be subject to greater velocities
- Other shoreline features that may create eddies
- The presence of nearshore structures, which tend to slow nearshore currents unless localized focusing effects or strong eddies are generated
- The general “roughness” and physical complexity of the bank surface.

Wind-generated wave action or vessel wakes can also cause bank erosion as these waves break on the shoreline and dislodge riverbank materials. Wave action can be diminished or augmented in particular areas due to concentration of reflected waves and/or the length of wind fetch to which the shoreline is exposed.

Erosion of relatively exposed bank material can occur in localized areas where stormwater sheet flow, particularly from nearshore impervious surfaces, flows to small low spots and becomes concentrated into rivulets or small streams. These flows can also cause saturation of bank soil and sediment, which may make them more unstable and susceptible to mass wasting.

<sup>29</sup> The OHWM refers to the upper edge of the riverbank and is defined as approximately +20 ft (NAVD88; DEQ 2003b). The OHWM defines the elevation beyond which inundation by the river is limited to extreme flow events, which occur approximately every five years.

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For riverbank materials to represent a potential loading term to a river, two conditions must be met: 1) the materials must be in a form that is potentially available for erosion into the river; and 2) ~~chemicals-contaminants~~ must be present at elevated levels within bank soils and sediment. Because most banks in the Study Area are longstanding, vegetated, and in equilibrium with common currents, boat wakes, and overland transport or runoff, mass wasting during extreme events may be the only significant loading process. For typical years, there is likely no significant load from riverbanks, as ~~indicated-discussed~~ below.

#### 6.1.7.1 Bank Materials Available for Erosion

Regardless of the ~~exact~~ force exerted on the bank, the degree of erosion generated is highly dependent on the physical conditions of the bank itself, the type of materials present, and how directly exposed the materials are to these forces. Primary factors affecting the susceptibility of banks to erosion include the following:

- Presence of protective and stabilizing vegetation (natural or planted)
- Presence of stabilizing structures such as bulkheads
- Presence of riprap, concrete, or other materials intended to protect the bank
- Steepness and overall profile of the bank
- Type of soils and sediment (e.g., consolidated, loose, gravel, sand, silt, cohesive clay, fill, or natural materials)
- Degree of saturation
- Presence of debris or artificial bank structures placed for purposes other than bank protection (e.g., boat ramps)
- Presence of docks, piers, dolphins, pilings, breakwaters, groins, and shoreline structures
- Presence of bench or beach areas below the bank, which can act to dissipate wave forces higher on the bank.

Where protective vegetation, structures, or materials are present, the type and condition of the materials underneath are often less important to erosion rates. An ODFW study (Vile and Friesen 2004) reviewed and inventoried the shoreline features from Willamette Falls (RM 26.5) to the confluence with the Columbia River (RM 0) and broke them into twelve general classifications based on nearshore habitat types. Using this data and several other sources of information, the City of Portland updated its natural resource inventory from the Broadway Bridge (RM 11.6) to the Columbia River, ~~and The City of Portland (2008d)~~ reported ~~(City of Portland 2008d)~~ the most common bank types within this area are vegetated riprap, sandy and rocky beach, and unclassified fill. Percentages of shoreline bank classification are as follows:

- Vegetated riprap—25 percent

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- Beach—23 percent
- Unclassified fill—21 percent
- Pilings limiting light—13 percent
- Non-vegetated riprap—12 percent
- Bio-technically engineered banks (artificial materials with vegetation aimed at bank stabilization)—3 percent
- Sea wall—2 percent
- Rock—1 percent
- Pilings allowing light—0 percent.

Map 3.1-17 maps these bank classifications, adapted from the 2004 ODFW (Vile and Friesen 2004) study, maps these bank classifications. Only if these classifications, only beach and unclassified fill, which together represent approximately 44 percent of the shoreline, are likely to be at all susceptible to erosion. Unclassified fill occurs in areas that were filled over time with a variety of unconsolidated materials and debris. No engineered riprap covers the surface, and banks can become unstable from erosive river forces and slump into the river. River beach areas are less steep, are in equilibrium with in-river physical processes, and can often act as a buffer, as noted above. Thus, unclassified fill (at 21 percent) is likely to be the most susceptible to erosion during extreme events. However, (This classification, however, represents a diverse range of physical conditions, and some of these areas have surfaces composed of various-sized rocks, sporadic vegetation, artificial debris of various types, and natural debris such as logs and wood, all of which may protect some of these areas to some extent.

For purposes of loading estimation purposes, approximately 15 to 25 percent of the shoreline is assumed to be potentially susceptible to erosion of bank materials. The low end of the range accounts for the presence of vegetation and natural and artificial debris that can reduce the potential for erosion. The high end of the range overestimates the amount of unclassified fill that is susceptible to erosion, but it allows for the fact that some portions of the other categories may include small areas that are relatively susceptible to erosion.

#### 6.1.7.2 Bank Material Chemistry

The LWG has searched documents available from DEQ through December 2007 for bank soil and sediment chemistry results associated with upland sites. Very little bank soil and sediment chemistry data are available in for Portland Harbor. Nearshore soil and sediment chemistry data from other parties were compiled from DEQ documents and uploaded to the LWG's SCRA database. The bank data contained in the SCRA database are summarized in Table E7-1, Appendix E, and are provided in full in the SCRA data file provided on CD in Appendix E Section 7.0 (Attachment E-3). The selection process for summarizing the bank data included the following:

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- Data collected between RM 1.9 and 11.8
- Data collected since May 1997
- Surface sediment or soil data (0–40 cm)
- Samples collected between 11 and 20 ft NAVD88
- Category 1 and 2 data.

The locations of the riverbank data points relative to the bank categories described above are shown in Map E7-1, Appendix E. As noted above, the table and map include samples that were taken near but not necessarily within the zone from +13 to +20 ft NAVD to capture as much bank data as possible. ~~It should also be noted that~~ riverbank soil and sediment data are available upstream of the Study Area in the downtown reach (Zidell South Waterfront property and Ross Island) and upriver (Oaks Bottom Landfill area and Willamette Falls). These data are not summarized in Table E7-1, but they are provided with the complete bank SCRA data file provided on CD in Appendix E Section 7.0.

~~The table and map show that a~~ considerable portion of the harbor riverbank materials that may be generally susceptible to erosion have no existing bank chemistry data. In these cases, it is impossible to quantify the concentrations and extent of ~~chemicals~~ contaminants that may be present and available for transport to the river via bank erosion. Among the sites with available bank chemistry data, ~~chemicals~~ contaminants on the ~~IC~~ loading lists were detected in bank soils and sediment that are potentially susceptible to erosion (Table E7-1).

#### 6.1.7.3 Riverbank Erosion Loading

Due to the paucity of existing bank condition and chemistry information at multiple shoreline sites, it is not possible at this time to ~~make even semi-quantitative estimates of~~ loading from this source to the river. Although it is estimated that approximately 15 to 25 percent of the banks within the Study Area are potentially susceptible to erosion, it is not possible to estimate typical erosion rates or a range of rates that might apply to these areas given the wide range of conditions present.

~~Bank loading estimates are not possible given the data limitations described above.~~ However, it is unnecessary to develop such loading estimates for the purposes of the RI/FS. Because bank erosion is an area-specific condition dependent on both the erodibility and ~~chemical~~ contaminant concentrations ~~of at~~ any given bank area, the potential role of bank erosion may need to be evaluated as a part of the future remedial design process for each sediment management area (SMA). ~~For the purposes of the FS, it will be assumed that potential bank erosion sources will be controlled before, or as part of, remedial action.~~

**Commented [awc71]:** This sentence should be retained: "Bank loading estimates are not possible given the data limitations described above." because it provides important information.

**Commented [KK72R71]:** This was stated in the first paragraph, so it does not need to be restated. Also, delete last sentence of this paragraph.

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### 6.1.8 Historical Loading

The previous sections focused on current loading to the Study Area; ~~however~~ However, historical loading is an important element of the CSM because ~~historical sources~~ it affect the current ~~IC-contaminant~~ IC-selected contaminants distribution. Historical loading is defined as any loading of ~~ICs~~ selected contaminants to the Study Area that occurred prior to collection of the data sets used to assess the current loading terms. Historical loading is reasonably expected to have occurred by all of the loading mechanisms assessed above: upstream surface water, bedload, stormwater, non-stormwater direct permitted discharges (as defined in Section 6.1.3), groundwater plume discharge, atmospheric deposition, riverbank erosion, and advective loading. For each mechanism, the historical load may be significantly greater than current load due to changes in regulations and reduction or elimination of chemical use in the Study Area and Willamette Basin. Additionally, historical records indicate that releases from overwater activities<sup>30</sup> were significant in the Study Area, though they are not specifically quantifiable with available records.

This section presents a qualitative discussion of historical loading to the Study Area, focusing on the expected relative load from historical versus current sources. This discussion complements the presentation of historical sources in Section 4. Information from both of these sections and from the subsurface sediment record are presented together in Section 10 (~~CSM~~) and evaluated to assess the current traces of existing contamination to historical sources.

**Upstream Surface Water Loading** – Historical surface water loading to the Study Area is relevant to the extent that associated suspended solids were deposited and remain within the Study Area. It is likely that historical loading via upstream surface water was significantly greater than current loading for many chemicals; due to widespread historical use of pesticides and herbicides, historical wastewater, sanitary and stormwater management practices, and generally more limited regulatory controls on many ICs.

**Upstream Bedload** – The relative contribution of historical ~~upstream sediment loading,~~ as compared to current upstream sediment loading, is uncertain. ~~Factors~~ Various factors, including historical dredging, ~~lack of information regarding historical versus current upstream bedded sediment concentrations,~~ sediment transport during flood events, and complex sediment deposition patterns, and the lack of information regarding historical versus current upstream bedded sediment concentrations make it difficult to predict or generalize about the duration and long-term impact of upstream sediment migration and loading.

<sup>30</sup> While improved BMPs are likely to reduce the occurrence of overwater releases significantly, it is acknowledged that current and future releases could occur. No attempt is made in this report to predict and quantify such releases as a current loading term, and no additional analysis of this term is planned for the RI/FS.

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**Stormwater Loading** – Stormwater loading to the Study Area is expected to have been higher historically, prior to implementation of upland stormwater runoff controls in some areas, control of CSOs, changes in chemical use/production/incidental production (PCBs, dioxins), and control of industrial discharges to storm drains and CSOs.

**Non-Stormwater Direct Discharge Loading** – Historical loading to the Study Area from industrial discharges, is likely to have been significantly higher than current loading prior to the adoption and regulation of discharge permits and controls, is likely to have been significantly higher than current loading.

**Upland Groundwater Plume Loading** – Upland groundwater plumes are generally the result of historical releases. Groundwater plume loading to the Study Area was more significant historically, prior to institution of groundwater controls. However, the transport time for chemicals-contaminants in upland groundwater to reach the river makes it difficult to predict or generalize about the timing of the peak historical loads.

**Atmospheric Deposition** – It is likely that overall atmospheric loading to the Study Area has decreased from historical levels due to widespread adoption by many countries of controls on chemical production, usage, and air emissions. As a result, atmospheric concentrations have decreased, but have not been eliminated entirely.

**Riverbank Erosion** – The historical composition and chemistry of bank materials—and thus the overall impact of historical bank erosion—is highly uncertainunknown. Nevertheless, chemical-contaminant loading to the Study Area due to bank erosion was likely more significant historically, prior to upland soil cleanups and installation of erosion controls in many areas, including riprap and sea walls.

**Overwater Releases** – Historical loading from overwater releases is expected to have been more significant historically, prior to improved BMPs. While available records do not support quantification of these historical releases, the current sediment record provides some information in known release areas (see further discussions in Section 10.2).

**Advective Loading** – The relative historical advective loading rate from subsurface sediments to surface sediments is uncertainunknown; however, assuming that subsurface sediment (as defined for the advective loadingthis analysis), generally reflects historical rather than recent releases to the Study Area, a comparison of surface and subsurface advective loading estimates (discussed in detail in Section 6.1.6.3) offers initial insight into the combined effects of historical loading to sediment from all sources. Subsurface advective loading for PCBs, PCDD/Fs, DDx, and PAHs, subsurface advective loading exceeds surface advective loading at the scale of the entire Study Area. This suggests that loading of these chemicals to the sediment bed, as indicated by the subsurface interval, was historically higher historically than currently.<sup>34</sup>

<sup>34</sup> It should be noted, however, that the total PAH loading rate estimate is dominated by LPAHs, which are much more mobile in groundwater than HPAHs. Based on this, Thus, it is possible that the subsurface advective

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**Commented [awc73]:** Do not delete footnote 30, which discusses LPAH vs HPAH mobility and implications to estimation of historical loading.

**Commented [KK74R73]:** If the information in the footnote is important, then it should be included in the text. The pertinent language from the footnote has been moved into the text.

The total PAH loading rate estimate is dominated by LPAHs, which are much more mobile in groundwater than HPAHs. Thus, it is possible that the subsurface advective loading of PAHs is influenced by ongoing (current) upland groundwater plume loading, in addition to historical PAH contamination of subsurface sediments

### 6.1.9 External Loading Summary

The project data sets and relevant literature were evaluated to assess current and historical external chemical loading terms to the Study Area. The following external loading terms were considered in this evaluation ~~(list developed in consultation with EPA; Integral et al. 2006, pers. comm.)~~—upstream loading via surface water (including suspended sediment load and sediment bedload), stormwater runoff, non-stormwater permitted discharges ~~(non-stormwater)~~, upland groundwater ~~(plume transport to the Study Area)~~, atmospheric deposition ~~(to the river surface)~~, upland soil and riverbank erosion, groundwater advection through sediments ~~(partitioning from deep sediment, transported by advecting to shallow sediment interval)~~, and overwater releases. Of these, a range of quantitative to semi-quantitative current annual loads to the Study Area were estimated for upstream surface water loading, stormwater runoff, permitted discharges (non-stormwater), upland groundwater plumes, atmospheric deposition, and groundwater advection through sediments. Historical loading is expected to have occurred by all of these loading mechanisms; ~~however, but~~ insufficient data are available to support quantitative estimates. Therefore, historical loading was only qualitatively ~~assessed qualitatively~~.

Commented [awc75]: Removal of “semi” makes this sentence nonsensical. Retain “semi-quantitative”.

Commented [KK76R75]: See response above.

Table 6.1-11 provides a summary of the central estimates of external current loading to the Study Area for upstream surface water, stormwater runoff, non-stormwater permitted discharge ~~(non-stormwater)~~, atmospheric deposition, upland groundwater plumes, and advection through subsurface sediments. The estimated annual loads for the internal transport<sup>32</sup>-mechanism of advection through surface sediments to surface water is also shown in Table 6.1-11 for comparison. For the 13 select indicator chemicals evaluated in the CSM (total PCBs, TCDD TEQ, total DDx, total PAHs, BEHP, total chlordanes, aldrin, dieldrin, arsenic, chromium, copper, zinc, and TBT), Section 10 provides additional summary information regarding the relative loads associated with each loading term, as well as the range of potential variability associated with each term.

Commented [awc77]: Retain the reference to Section 10. Also retain footnote 32. This information provides important context for the reader to understand why this work was performed, and how it influences later portions of the RI and FS.

Commented [KK78R77]: Section 10 will not include any new information. If it is not discussed here, it will not be presented in Section 10. If information in the footnote is important, then need to include in the text. Not informative in a footnote.

loading of PAHs is influenced by ongoing (current) upland groundwater plume loading, in addition to historical PAH contamination of subsurface sediments.

<sup>32</sup>Groundwater advection is the only internal process that is quantified in this RI. The other internal processes, including sediment resuspension/transport/deposition, solid/aqueous phase partitioning, abiotic/biotic transformation and degradation, and biological uptake, may be significant in the transfer of chemicals from abiotic media to biota, and many of these terms will be quantified as part of the FS fate and transport modeling effort.

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## 6.2 FATE AND TRANSPORT

This section describes the physical, chemical, and biological processes that influence the fate and transport of contaminants within the in-water portion of the Study Area. This discussion of fate and transport processes is organized in three main subsections, corresponding to the major environmental compartments of the Study Area: 1) surface mixed sediment layer and associated pore water, 2) surface water, and 3) biota. Figure 6.1-1 presents a conceptual drawing of these major environmental compartments.

For each of these three compartments, the relevant fate and transport processes are discussed for each of the chemical groups in the combined list of loading ~~ICs~~ contaminants (Table 6.0-1). These include PCBs, PCDD/Fs, pesticides (DDx and non-DDx), PAHs, petroleum hydrocarbons, SVOCs, metals, TBT, and VOCs. Insights into fate and transport processes gained from assessment of empirical information and the sediment physical transport modeling (~~FS~~-HST modeling) are also discussed. Due to project timing ~~and under agreement with EPA~~, results from the ongoing fate and transport modeling effort will be presented in ~~separate FS-related deliverables~~ the FS.

### 6.2.1 Sediment and Pore Water Fate and Transport Processes

The following subsections discuss fate and transport processes relevant to ~~ICs~~ select contaminants in the sediment and pore water<sup>33</sup> environment. General discussion of organic and inorganic chemical behavior in sediment and pore water is presented, followed by discussion of physical transport processes for these media. Insights from empirical data and from ~~FS~~-HST model results, where applicable, are also presented to help assess the relative importance of the various processes for the chemicals on the combined loading ~~IC-contaminant~~ list.

#### 6.2.1.1 Chemical Contaminant Distribution between Sediment and Pore Water

In the sediment and pore water environment, the distribution of a chemical between the solid and aqueous phases is among the most important physiochemical processes affecting its migration, bioavailability, and half-life. The equilibrium distribution of a chemical between water (dissolved aqueous phase) and solid (sorbed to sediment or associated organic matter) is ~~often~~ generally described by a solid/water distribution coefficient ( $K_d$ ).

$$K_d \sim (C_s/C_w)$$

Where,

$C_s$  = the concentration of the chemical associated with solids

**Commented [awc79]:** Retain the  $K_d$  equation and  $C_s$  and  $C_w$  definitions. O.k. to delete footnote 34 if trying to streamline, but this is important information and is not presented in App E.

**Commented [KK80R79]:** This is a common term that can be looked up in any reference document and is not unique to this analysis. It is not necessary to define it here.

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<sup>33</sup> The general term "pore water" is used here instead of TZW to acknowledge that the discussion also includes interstitial water in the sediment, which does not contain upland groundwater.

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$C_w$  = the aqueous concentration of the chemical<sup>34</sup>

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Major processes and environmental factors that control this distribution (which varies in response to environmental conditions such as pH, temperature, and salinity) are discussed below in general terms for organic and inorganic analytes. Observed partitioning ratios in the LWG-collected TZW and paired sediment data sets are compared to published literature values; for relevant analyte groups; for additional perspective. Finally, degradation and transformation mechanisms for chemicals in the sediment/pore water environment are also discussed.

#### 6.2.1.1.1 Organic Chemicals Contaminants

For organic analytes, the  $K_d$  term describes the combined effect of all possible equilibrium partitioning mechanisms affecting distribution between sediment and pore water, including hydrophobic sorption onto organic matter associated with the sediment, electrostatic attractions of oppositely charged ionic functional groups, and covalent bonding or complexation of ionic organic molecules with reactive surface groups. For nonionic organic chemicals, which include most of the organic chemicals on the IC loading list (PCBs, pesticides including DDx, PCDD/Fs, PAHs, SVOCs, and VOCs), the primary mechanism defining the  $K_d$  term is hydrophobic sorption onto organic matter. Therefore, for nonionic organic chemicals,  $K_d$  describes two-phase partitioning<sup>35</sup> to the organic matter on the solid surfaces and is a function of the tendency of the chemical to sorb to organic carbon ( $K_{oc}$ ) and the fractional organic matter content of the solids ( $f_{oc}$ ).

$$K_d = (K_{oc} * f_{oc})$$

Where,

Commented [awc81]: Note that the info presented in this footnote is not in App E.

Commented [KK82R81]: Noted.

Commented [awc83]: Retain the  $K_d$  equation and  $K_{oc}$  and  $f_{oc}$  definitions below.

Commented [KK84R83]: This is a common term that can be looked up in any reference document and is not unique to this analysis. It is not necessary to define it here.

<sup>34</sup> The rigorous definition of  $K_d$  includes the ratio of the thermodynamic activity coefficient of the chemical sorbed to the solids to the thermodynamic activity coefficient of the chemical in the aqueous phase. For equilibrium environmental conditions (generally low concentrations relative to the water and solids), activity coefficients approach 1, and the  $K_d$  is considered equal to the ratio of the chemical concentrations in sediment to the chemical concentrations in the aqueous phase.

<sup>35</sup> All of the sorption estimates presented in the RI assume two-phase partitioning. Two-phase partitioning of the chemical between water and organic matter neglects a potential third phase that may be present in the sediment environment, namely free or residual hydrocarbons. Based on field observations and a review of the sediment core data, evidence of free or residual hydrocarbons in the sediments is minimal. Out of 1,086 sediment cores collected in the Study Area, 35 core logs (or approximately 3 percent of the logs) made reference to visual observations of TPH or tar, indicating the potential presence of free or residual hydrocarbons in the sediments at these locations. Of these 35 cores, 18 cores were collected between RM 6 and 7, predominantly near the Koppers, Gaseo, and Siltronic sites. Because field observations do not indicate widespread presence of free or residual hydrocarbons in the sediments, the assumption of two-phase partitioning was deemed adequate to capture the predominant mechanisms affecting chemical distribution between sediments and pore water across the Study Area.

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$K_{oc}$  = the coefficient of partitioning between water and organic matter<sup>36</sup>

$f_{oc}$  = the mass fraction of organic carbon in the solids.

In addition to temperature, several factors can affect equilibrium partitioning behavior for nonionic organic chemicals:

- **Salinity** – High-salinity environments (e.g., seawater) can cause increased adsorption (decreased solubility and higher observed  $K_d$  than predicted at lower salinity). This may be relevant in the highly saline sediment and pore water environment local to offshore areas on the west side of the river, between roughly RM 7 and 7.5, where pore-water salinities in excess of typical seawater have been observed. It is unlikely to be a significant factor elsewhere in the river.
- **Cosolvents** – The presence of miscible organic liquids in solution with hydrophobic chemicals can result in increased solubility (and therefore decreased  $K_d$ ) of the hydrophobic chemical. This effect, however, requires significant amounts of cosolvent chemicals in solution (more than 10 percent by volume [Yalkowsky et al. 1976]).
- **Colloids** – Colloids are organic and/or inorganic particles in the system defined by their behavior (tendency to remain dispersed in water, not settle rapidly, and not filter easily) and size (usually 1 nm to 1  $\mu$ m in diameter [Lyklema 1991]). Colloids represent a portion of the surface area available for sorption of organic chemicals. Because colloids can be mobile in water within a sediment matrix, they can increase the “apparent” concentration of the hydrophobic chemical in the aqueous phase. Because colloids are <1  $\mu$ m in diameter, they would be included in both filtered and unfiltered water samples.
- **Characteristics of natural organic matter** – The nature of the organic matter present in the sediment can also affect the extent of partitioning, making partitioning behavior variable across different environments.

TPH, as analyzed for the RI, is the measure of all hydrocarbons and non hydrocarbons that can be quantified in the carbon range from  $n$ -C<sub>6</sub> to  $n$ -C<sub>40</sub>. Further, the components (TPH-diesel, TPH-residual, and TPH-gasoline) are simply descriptive laboratory terms for the fractions of TPH (and do not represent source assignments or indications of toxicity). TPH therefore represents a vast mixture of hydrocarbon chemicals, of both natural and anthropogenic origin, with an equally vast range of partitioning behaviors. As such, its behavior as a chemical group cannot be accurately characterized by a  $K_{oc}$  value. Because the subcomponents of TPH are

Commented [A85]: C12 and above seems outside the range of gasoline hydrocarbons. Are they included here or not?

Commented [KK86]: These are incorrect references to these contaminants.

Commented [awc87]: Text corrected to C<sub>6</sub>; GRH are included.

Commented [KK88R87]: OK

<sup>36</sup>  $K_{oc}$  is organic-matter specific; however, it is often generically calculated from empirical expressions based on the octanol-water partitioning coefficient ( $K_{ow}$ ) for a given chemical, which describes partitioning of the chemical between water and highly non-polar liquid octanol.

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unknown for all sampling results, the subcomponents also cannot be accurately characterized by  $K_{oc}$  values.

The only definitively ionic organic compound on the combined IC loading list is TBT. The partitioning behavior of TBT is strongly affected by pH and the identity of anions in solution that pair with the TBT ion (Arnold et al. 1997). Measured log  $K_{oc}$  values are specifically, for on the order of 4 at pH 10 to 7 the measured log  $K_{oc}$  values are on the order of 4, and; approximately 2 from at pH 7 to pH 3, they drop to roughly 2, corresponding to 2 orders of magnitude of a substantial behavioral variability as pH varies across the pH 7 boundary. The mean surface water pH is 7.38 (10<sup>th</sup> percentile is 6.98 and 90<sup>th</sup> percentile is 7.76). The observed pore water pH values measured in the GWPA ranged from 5.6 to 8.1. TBT in its nonionic form (not likely observed in the Study Area) is highly hydrophobic, with  $K_{ow}$  values on the order of 7 ( $K_{ow}$  WIN).<sup>37</sup>

Ranges of literature equilibrium partitioning values for organic chemicals on the advective loading IC contaminant list, representing the subset of chemicals for which equilibrium partitioning assessment is relevant, were compiled for the advective loading analysis (discussed in Section 6.1.6) and are presented in Appendix E, Table E6-5. The average range in the  $K_{oc}$  values for organic analytes is 1 order of magnitude, with PCDD/Fs, TBT, and BEHP showing exhibiting a range of more than 2 orders of magnitude, representing substantial variability in partitioning behavior, representing significant variability/uncertainty in partitioning behavior.

Limited site-specific empirical information, consisting of to assess sediment pore-water partitioning of organic chemicals is limited to the filtered TZW data set with paired surface sediment samples, is available to assess sediment pore water partitioning of organic chemicals. This data set is limited as a representation of the Study Area because it focuses only on the offshore area of only the TZW study sites, (the TZW investigation is described in detail in Section 5.4), and not all chemicals-contaminants of interest in sediment were analyzed in TZW samples (e.g for example, PCBs were not analyzed in any TZW samples, and DDx and PCDD/Fs were analyzed in only a small fraction of the samples). However, because it is the only available empirical data-set, observed partitioning values were plotted against the corresponding literature partitioning values. Observed partitioning for PAHs and DDx pesticides between filtered TZW and sediment are shown in Figures 6.2-1 and 6.2-2, respectively. These figures also show the literature values compiled for use in the advective loading assessment (Section 6.1.6). Results for DDx show exhibit a wide range of literature partitioning coefficients cited in the literature for these analytes. Comparisons of the literature-derived partitioning values for DDx pesticides with observed partitioning behavior are limited by the small number of sample pairs ( $n=4$ ) for which a given isomer was detected in both TZW and sediment. This limited set of observed partitioning values also spans a broad range. In contrast, PAHs (Figure 6.2-1), in

Commented [awc89]: This text makes an important point and should be retained: "representing significant variability/uncertainty in partitioning behavior".

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<sup>37</sup> KowWIN software available online as part of EPA Estimation Program Interface suite of programs: <http://www.epa.gov/oppt/exposure/pubs/episuite.htm>.

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~~contrast, show the range of partitioning coefficients obtained from literature sources for individual PAHs (Figure 6.2-1) exhibits a relatively narrow range; tighter range of literature partitioning coefficients for most individual compounds, while the observed partitioning shows much wider ranges, especially for the LPAH constituents. The wide variability in observed partitioning for PAHs may reflect multiple factors, possibly including non-equilibrium conditions between TZW and sediment, small-scale spatial variability (sediment and TZW sample pairs were not perfectly exact; always well co-located), and/or filtered samples not reflecting truly dissolved concentrations (e.g., effects of colloidal fractions smaller than the 0.45 µm filter size).~~

Commented [awc91]: Prefer the term "exactly" to "well" because "exactly" is more specific.

Commented [KK92R91]: Changed to always.

#### 6.2.1.1.2 Inorganic ~~Chemicals~~Contaminants

The fate and transport of inorganic species (~~e.g., metals~~) in pore water is defined by the distribution ~~of the species~~ between the aqueous and solid phases. A wide range of mechanisms control the distribution of metals between ~~the aqueous and solid~~ these phases. ~~Most commonly these mechanisms include~~ precipitation/dissolution reactions and sorption/ion-exchange processes. Precipitation and dissolution are controlled by the concentration of species present both in solution and as mineral phases. Sorption and ion exchange are controlled by a variety of factors, including electrostatic attraction, covalent bonding, and weak intermolecular attractions such as van der Waals forces.

The distribution of inorganic species between the aqueous and solid phases is controlled by a number of mechanisms that are a function of the physical, chemical, and biological characteristics of the solid-aqueous system. The characteristics most important for the aqueous solution phase include the following:

- pH
- Oxidation-reduction potential (Eh)
- Presence of competing ions
- Aqueous complexation reactions
- Ionic strength and the specific ions in solution.

The solid phase characteristics of importance include the following:

- Grain size
- Composition/mineralogy
- Sorbed organic carbon content and type
- Surface characteristics such as charge, coatings, and area.

In addition, there is a range of factors that cannot easily be assigned to one phase, such as temperature and the fugacity of gases such as oxygen and carbon dioxide.

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The aqueous-solid chemistry of the sediment and pore water environment can be strongly influenced by microbial processes. Microbial oxidation of labile organic carbon, ~~both natural and anthropogenic~~, frequently depletes dissolved oxygen in pore water, resulting in chemically reduced conditions and the production of alkalinity. Further, under anaerobic conditions, microbial processes can induce numerous environmentally relevant changes to the chemical environment, such as dissolution of iron and manganese oxide minerals and production of sulfides.

Sorption and ion-exchange mechanisms for metals can empirically be described by the solubility constant  $K_d$ . Unlike organic chemicals, the appropriate  $K_d$  term is not a function of  $f_{oc}$  (although organic matter can also sequester inorganic chemicals, thereby affecting the  $K_d$  value). Literature  $K_d$  values were compiled for the metal advective loading ICs: arsenic, copper, lead, and mercury. These values were used in the calculation of pore water concentrations for estimation of loading to surface water from surface sediment via groundwater advection. This analysis and the significant uncertainty associated with the inherent assumptions are presented in Section 6.1.6, and the range of literature  $K_d$  values is presented in Appendix E, Table E6-6. These values show ranges of 1 to 3 orders of magnitude. The wide range in literature  $K_d$  values for metals reflects the strong, highly variable geochemical factors described above that influence partitioning behavior in environmental systems. Considering this, literature  $K_d$  values should be considered site-specific estimates resulting from the geochemical conditions particular to individual studies. Limited site-specific empirical information, consisting of the filtered TZW data set with paired surface sediment samples for arsenic, copper, lead, and mercury, is presented for general comparison purposes on Figure 6.2-3.

A basic understanding of the fate and transport of inorganic species requires an understanding of the saturation state of aqueous chemical species in the system with respect to the specific components of minerals that are composed of those species and that may precipitate or dissolve, depending on the saturation state whether the specific mineral species is likely to remain in solution or form a precipitate. For a given mineral phase salt, the solubility product,  $K_{sp}$ , defines the dynamic equilibrium condition between dissolved ions and the corresponding mineral. Generically, for the dissolution of a hypothetical mineral  $A_bC_d$ :

$$K_{sp} = [a]^b * [c]^d$$

Where,

[a] and [c] = the concentrations<sup>38</sup> of ions a and c in solution

<sup>38</sup> Rigorously, this refers to the activity of the metal in solution, which refers to the product of the thermodynamic activity coefficient and the concentration. For equilibrium environmental conditions (generally low concentrations relative to the water and solids), activity coefficients approach 1, making the concentration a good approximation of the product.

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b and d = the stoichiometric coefficient of each ion that is present in mineral phase ( $A_bC_d$ ).

An excess of the ion activity products (i.e., right hand side of the equation) Higher concentrations of the component ions in solution relative to  $K_{sp}$  favors the precipitation of the mineral from solution, while a deficit of the ion activity products in lower concentrations solution favors the dissolution of the mineral to solution (provided the mineral phase is present). The solubility product defines the aqueous solid phase conditions solubility under thermodynamic equilibrium. However, many reactions are kinetically limited and thus it is not uncommon for non-equilibrium conditions to exist in natural aqueous systems.

Sorption and ion-exchange mechanisms for metals can empirically be described by the  $K_d$  relationship described above; however, unlike ~~Unlike~~ for organic chemicals, the appropriate  $K_d$  term is not a function of  $f_{oc}$  (although organic matter can also sequester inorganic chemicals, thereby affecting the  $K_d$  value). Literature  $K_d$  values were compiled for the metal advective loading ICs: arsenic, copper, lead, and mercury. These values were used in the calculation of pore water concentrations for estimation of loading to surface water from surface sediment via groundwater advection. This analysis and the significant uncertainty associated with the inherent assumptions are presented in Section 6.1.6. ~~and the range of literature  $K_d$  values exhibit ranges of 1 to 3 orders of magnitude, and are~~ is presented in Appendix E, Table E6-6. ~~These values show ranges of 1 to 3 orders of magnitude. The wide range in literature  $K_d$  values for metals reflects the strong, highly variable geochemical factors, described above, that influence partitioning behavior in environmental systems. Considering this, literature  $K_d$  values should be considered site specific estimates resulting from the geochemical conditions particular to individual studies. Limited site specific empirical information, consisting of the filtered TZW data set with paired surface sediment samples for arsenic, copper, lead, and mercury, is presented for general comparison purposes on Figure 6.2-3.~~

Commented [awc93]: This is useful information and is not in App E. However, it is not absolutely essential.

Commented [KK94R93]: This is a common concept that can be looked up in any reference document and is not unique to this analysis. It is not necessary to define it here.

#### 6.2.1.2 Degradation/Transformation Processes

A variety of abiotic degradation/transformation reactions, including hydrolysis, dehalogenation, oxidation, and reduction, can occur in aqueous systems. Hydrolysis is a well known chemical ~~a~~ reaction by which alkyl halides, esters, or ester analogs are split, and hydrogen and hydroxide (the components of water) are added to produce ~~converted to~~ alcohols or organic acids (or both in the case of esters). Dehalogenation ~~refers to are~~ reactions in which ~~ehlorine halogen~~ atoms (such as chlorine) are removed ~~from from alkyl halides~~ halogenated hydrocarbons. Oxidation and reduction are complementary reactions that involve the loss of one or more electrons (oxidation) by one chemical species and the gain of one or more electrons (reduction) by another. Metals in environmental systems are subject to both oxidation and reduction reactions, depending on the particular metal, its speciation in the environment, and other geochemical conditions. ~~Of the organic chemicals contaminants on the combined IC-contaminant loading list, PAHs, PCP, TPHs (aromatic and~~

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aliphatic components), DDX, and aldrin are subject to degradation/transformation by abiotic processes in the sediment/pore water environment—, though the degradation rates are relatively slow for PCBs, BEHP, hexachlorobenzene, 1,2-DCB, chlordanes, dieldrin, and dioxins are also subject to abiotic transformation/degradation; however, abiotic processes in sediment and pore water are very slow for these chemicals. Many of the IC chemicals are subject to photo-oxidation in of contaminants present in surface water, including PCBs (minor), dioxins (when near the water surface), aldrin, PAHs (especially LPAHs), PCP, and TBT, as is discussed in Section 6.2.2.3.

Biodegradation is likely can be a significant process for some certain various types of organic compounds contaminants found in Study Area sediments and pore water in the Study Area. Biodegradation It involves the metabolic oxidation or reduction of organic compounds and is carried out predominantly by bacteria in aqueous environments, but yeasts and fungi may also contribute to biodegradation. In general, oxidation of organic compounds occurs under aerobic conditions and reduction under anaerobic conditions, although both processes can occur under both conditions. Microbes may either gain chemical energy directly as a result of biodegradation of an organic compound, or during the process of co-metabolism, the concurrent degradation of another substrate with the organic compound. The chemicals on the IC list that can typically be transformed or degraded by microbially mediated processes include DDX, gamma-HCH, TBT, PCP, aldrin, PAHs, and to a lesser extent PCBs, BEHP, and hexachlorobenzene. MO of the metals on the IC list, microbially-mediated transformations of metals are is only significant for mercury<sup>39</sup> and lead<sup>40</sup> organocompounds.

The biodegradation rates depends on the chemical structure and concentration of the organic compound, the concentration of bacteria responsible for the biodegradation, the availability of organic matter to serve as food and energy sources for bacterial growth, and physical and chemical conditions at the site, such as temperature and oxygen level. The extent to which the organic compound is bound to particles may also affect the

Commented [awc95]: Retain text. It is useful to know which chemicals may be impacted by microbial processes.

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<sup>39</sup> Mobilization of sorbed mercury can be caused by bioreduction to elemental mercury and bioconversion to more volatile and soluble forms, such as methylmercury. Methylmercury is the most hazardous mercury species due to its high stability, its lipid solubility, and its possession of ionic properties that allow it to readily pass through cellular membranes (Eisler 1987). Mercury discharged into rivers, bays, or estuaries can be converted into methylmercury compounds by natural biological (bacterial microorganisms) or chemical processes (Eisler 1987). The mercury methylation process depends on mercury loadings, microbial activity, nutrient content, pH and redox condition, suspended sediment load, sedimentation rates, and other variables; anaerobic conditions favor methylmercury formation more than aerobic conditions (Eisler 1987). Bacterial microbes are also responsible for methylmercury decomposition (demethylation).

<sup>40</sup> Of the organoleads, tetraethyllead and tetramethyllead are the most stable organoleads, and the most important because of their widespread use as antiknock fuel additives. Both are clear, colorless, volatile liquids, and highly soluble in many organic solvents; however, Solubility in water is only 0.18 mg/L for tetraethyllead, and 18.0 mg/L for tetramethyllead. Both undergo photochemical degradation in the atmosphere to elemental Pb and free organic radicals, although the fate of automotive organoleads has yet to be fully evaluated (Eisler 1988). In general, organolead compounds are more toxic than inorganic Pb compounds, food chain biomagnification of Pb is negligible, and younger organisms are most susceptible (Eisler 1988).

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biodegradation rate ~~because as~~ the bound organic compounds may be biologically less available for microbial uptake. ~~Biodegradation models commonly used include the Michaelis-Menten equation and the Monod equation, which account for chemical concentrations, microbial density, changes in uptake rates with changes in concentration, and changes in microbial density over time. By assuming constant biodegradation rates and bacterial density, the power rate law can be used to describe biodegradation empirically:~~

$$dC/dt = kC^n$$

Where,

C = chemical concentration

t = time

k = rate constant for biodegradation, determined experimentally in the laboratory or measured in the field

n = 0 for zero order kinetics and 1 for first order kinetics.

A wide variety of microbial species that ~~use a large number of~~utilize different biochemical pathways to metabolize anthropogenic ~~chemicals-contaminants~~ have been identified. Biodegradation can proceed to full mineralization ~~of the compound~~ with end products of carbon dioxide and water, or an intermediate compound may be formed that is not easily ~~further~~ biodegraded ~~further~~. ~~For example DDT; for example, is relatively readily~~ biodegraded to DDE ~~relatively easily~~, but DDE is more persistent. The susceptibility of organic compounds to biodegradation depends on several factors, such as the presence and type of functional groups (oxygen- and nitrogen-containing groups increase biodegradation rates), the size and chemical structure of the organic compound (small molecules biodegrade more readily than large molecules), and solubility (more soluble organic compounds biodegrade more readily). A literature review has been completed to find appropriate biodegradation rate constants for use in the Fate and Transport Model; ~~these were compiled in cooperation with EPA and will be presented in the final RI and discussed in the context of fate and transport findings.~~

### 6.2.1.3 Sediment Physical Transport Processes

Hydrophobic chemicals are strongly associated with sediment particles, in particular cohesive or fine-grained particles (silts and clays). As a result, the physical transport of sediments, especially silts and clays, will affect the distribution and fate of hydrophobic ICs within the Study Area. Many of the Portland Harbor ICs (e.g., Hydrophobic contaminants found in Portland Harbor include PCBs, PCDD/Fs, pesticides including DDT compounds, organochlorine pesticides, and PAHs) are hydrophobic chemicals strongly associated with sediment particles, in particular cohesive or fine-grained

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particles (silts and clays); therefore, the physical transport of sediments, especially silts and clays, may impact the distribution and fate of certain ICs within the Study Area.

Sediment movement into, within, and through the Study Area occurs as suspended load in the water column and as bedload along the riverbed. Cohesive or fine-grained sediments generally move as suspended load, which is defined as suspended particulate transport in the water column. Cohesive or fine-grained sediments generally move as suspended load. Noncohesive sediments (sands and coarser) typically move as bedload. Bedload transport, of sediment refers to which refers to sediment transported along or very close to the riverbed. H (as described in Section 6.1.1.2). Noncohesive sediments (sands and coarser) typically move as bedload; however, a variable fraction of noncohesive sediments move as suspended load as a function of the flow regime (i.e., as flows increase, a larger fraction of noncohesive sediment will move in suspension).

The movement (resuspension and down gradient suspended load and bedload transport) of sediments in the Lower Willamette River is controlled by the both natural and anthropogenic forces that affect water movement and the bottom shear stresses. As noted-discussed in Section 3, natural flow regimes vary-exhibit a wide range dramatically (typically 10-20x annually) between the dry summers and rainy winters in Portland Harbor. Based on empirical, site-specific erosion velocities measured with Sedflume and modeled bottom shear stress, significant natural resuspension and movement of sediments does not generally occur at river flows below approximately 40-50,000 cfs (Section 3.1.4.2.3). Late spring through fall Lower Willamette River flows are typically well below this level (see Figure 3.1-4), whereas late fall and winter flows exceed this threshold for variable lengths of time, depending on the intensity of winter precipitation events in the Willamette Basin. This strong seasonal pattern in dynamics applies to the deep channel environment in the Lower Willamette River which, on an areal basis, makes up much of the riverbed. Flows in nearshore, off-channel areas are severely dampened by nearshore structures, bottom drag, and shoreline configuration (e.g., sheltered embayments and slips). As shown on Map 3.1-11, shows that modeled bottom shear in many off-channel areas remains relatively low even during a river flow event of 160,000 cfs. This pattern is important because most of the areas of relatively high sediment contaminant concentrations in Portland Harbor are located in off-channel areas (Section 5.1).

In contrast to the channel environment, sediment disturbance/resuspension/scour in nearshore areas, particularly around working piers, berths, marine terminals, and others areas with significant boat traffic, may be largely a function of anthropogenic factors, such a prop wash, rather than natural factors alone. This effect may be accentuated during low flow portions of the year (late summer/fall) when river stage is also low (Figure 3.1-3) reducing vessel drafts. This anthropogenic influence is suggested by the time-series bathymetric measurements (Map 3.1-7), which reveal scour patterns in sheltered areas, such as Swan Island Lagoon and the International Slip, as well as very

Commented [GR97]: Revised numbering in Final Section 3, this is now Section 3.1.5.2.3

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close to shore in portions of the main stem that do not experience high bottom shear forces even during high river flows.

As described in Section 3.1 and summarized in Table 3.1-5<sup>11</sup>, the physical character of the Lower Willamette River transitions rather abruptly near the upstream end of the Study Area (about RM 10) from a relatively narrow, high velocity river characterized by coarse-grained riverbed channel sediments upstream to a broader, slower river dominated by fine-grained sediments downstream. This relatively wide, fine-grained character extends from RM 10 to the lower end of the RI Study Area at RM 1.9, with the exceptions of a distinct, narrow, higher energy reach between RM 5 and 7 and a small area at the head of the Multnomah Channel; both of these areas are dominated by sands (Map 3.1-2). Map 3.1-12 illustrates these overall patterns in surface grain size gradients. Measured areas of sediment scour and deposition from 2002 to 2009 (time-series bathymetry), and modeled predicted bed change during a major flood events are shown on Map 3.1-12, and depict. The three panels on Map 3.1-12 show consistent areas of erosion and deposition in the Lower Willamette River during both typical (observed) and extreme (modeled) flow conditions. These areas correspond to sand-dominated (erosion) and silt-dominated (deposition) reaches. Again, this pattern applies to the deeper, in-channel portions of the river and appears to reflect the influences of natural forces. Nearshore areas are subject to a more complex mix of natural forces and smaller-scale, anthropogenic factors, such as vessel traffic, river stage variations, and in-water construction/dredging and fill activities that affect localized sediment texture and resuspension/transport patterns.

Commented [GR98]: Same as previous, table renumbered in Final RI Section 3 revision

Within the general framework of Study Area sediment transport regimes summarized above and detailed in Section 3.1, the major transport and fate processes relevant to sediment-bound chemicals are sediment transport into the Study Area from upstream, downstream sediment migration out of the Study Area (either in the main stem or Multnomah Channel), and the fate and transport of sediments within the Study Area, such as surface sediment mixing and resuspension, permanent burial at depth in the sediment column, and biological uptake. These processes are addressed below.

#### 6.2.1.3.1 Sediment Flux into/out of the Study Area

Sediment enters Portland Harbor as suspended load carried in the water column and as bedload transported along or very close to the riverbed. Suspended and bedload sediment fluxes are discussed separately below.

##### Suspended Sediments

A significant volume of suspended sediment data has been collected in the Lower Willamette River across a range of hydrologic conditions both by the LWG, as part of the surface water sampling program and as a HST model need, and by other entities (e.g., the City of Portland), as part of long-term monitoring programs. These data are described in Section 3.1.5.2.4.4-3 and show that suspended loads are strongly correlated with flow and accordingly vary widely (from approximately 5 to 50 mg/L) seasonally and annually (see Figures 3.1-46<sup>16</sup> and 3.1-48a<sup>25</sup>18a-h). The data also suggest

Commented [GR99]: Changes to figure numbering and content from Final Section 3

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Higher suspended loads ~~isare observed~~ on the rising limbs of the hydrographic events than on the falling limbs. Finally, a series of *in-situ* suspended particle size measurements conducted for the hydrodynamic modeling data collection effort indicate that suspended sediment particles sizes are comparable throughout the Study Area, with a median percentile particle diameter between 15 and 30  $\mu\text{m}$  (silt). In contrast, the median grain-size diameter at an upriver location (RM 18) was 78  $\mu\text{m}$  (fine sand), reflecting the higher energy environment in the upper Lower Willamette River (Figure 3.1-17~~29~~).

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The modeled ~~(FS-HST)~~ suspended load fluxes into and out of the Study Area ~~(entering at RM 11.8 and leaving in the main stem at RM 1.2 and down Multnomah Channel)~~ are included in Table 6.1.3. Across the modeled flow years (5<sup>th</sup>, 25<sup>th</sup>, 50<sup>th</sup>, 75<sup>th</sup>, and 95<sup>th</sup> percentiles), average total suspended sediment flux into the Study Area from upstream equals about 1.53 billion kg/yr, and the average total suspended sediment flux out of the Study Area equals about 1.26 billion kg/yr, indicating a net accumulation of about 0.28 billion kg/yr in the Study Area. ~~A~~The model suggests, therefore, that averaged over time, about 18 percent of the suspended material entering the harbor accumulates somewhere between RM 11.8 and 1.2. ~~As discussed in Section 3.1.4.2 and presented in Table 3.1-6, the~~The average annual net sediment accumulation rate calculated from empirical bathymetric survey data collected between 2003 and 2009 was 0.20 billion kg/year, which is in very good agreement with the model estimates, especially the 50<sup>th</sup> percentile model estimates, ~~of sediment influx and efflux,~~ which correspond to a net accumulation of 0.19 billion kg/year. Net sediment accumulation represents a combination of new material entering the Study Area from upstream and some percentage of bedded sediment that is resuspended from the riverbed within the Study Area and then redeposited further downstream, but before exiting the Study Area. ~~The relative magnitude of these suspended loads, their sediment quality, and the spatial distribution of deposition and resuspension over time determine the potential for recontamination/natural recovery in the harbor.~~

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### Bedload Sediments

Noncohesive bedload sediments move downstream along (rolling, sliding) or just above the riverbed (saltation) whenever near-bottom shear stresses exceed the threshold for sediment movement. Unlike suspended load, no direct measurements of bedload have been made as part of the RI/FS, ~~as because~~ it is extremely difficult to measure in the field. Additionally, ~~as discussed previously,~~ bedload is assumed to be a minor sediment transport mechanism due to the generally cohesive character of the sediment bed of the Lower Willamette River. ~~In light of that, and in consideration of the presumption that contaminants are not strongly associated with coarse grained sediments~~Consequently, bedload processes are not quantified in the RI/FS fate and transport evaluation.

#### 6.2.1.3.2 Sediment Fate and Transport in the Study Area

The spatial pattern and extent of deposition and erosion in the Study Area was measured directly with the time-series of precision bathymetric surveys conducted from 2002 to

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2004 (described in [Section 3.1.5.2.4.2](#)). ~~As noted above,~~ The bathymetric change data show distinct zones of deposition and erosion that reflect the cross-sectional area and resultant hydrodynamics of specific portions of the Study Area (see Maps 3.1-10 and 3.1-98). These hydrodynamic reaches, described in [Section 3.1.5.2.6](#), share attributes such as sediment texture and general sediment stability.

Commented [GR102]: Section 3 renumbered

Based on surface and subsurface grain-size (percent fines) distribution (Maps 3.1-32 and 3.1-54) and bathymetric features (Map 3.1-95), areas of fine-grained sediment accretion appear to be dominant from RM 8 to 10, along the channel edge from RM 4 to 5, and from RM 1.5 to 3. The downstream and upstream areas noted here are known to be long-term sediment accumulation areas based on historical dredging needs. Upstream depressions (borrow pits) between RM 9.5 and 11, that in combination span the navigation channel, likely capture some suspended and much of the bedload (noncohesive) sediments that are entering the system. The Study Area reaches between RM 5 and 7 and RM 10 and 11.8, where the river is relatively narrow, are dominated by areas of small-scale net erosion, as is the western off-channel area from RM 0 to 3 (outside bend of the [Lower Willamette River](#) as it turns toward the Columbia).

~~The~~ analysis of the time-series bathymetric change data presented in [Section 3.1.5.4.2.2](#) indicates that during typical flow conditions only about 10 percent of the riverbed exhibited net bathymetric changes (erosion or accretion) greater than 30 cm, but that relatively “small-scale” scour or accretion from about 8 cm (the limit of bathymetric resolution) to 30 cm in extent was widespread. ~~These data suggest,~~ possibly indicating that the top 30 cm of the sediment column is relatively unconsolidated and more susceptible to resuspension/erosion than deeper sediments.

As described in ~~Section 5~~ [Appendix E](#), ~~Appendix H 4.2.2~~, radioisotope cores were collected in upstream borrow pits at RM 10.5 and 10.9 in an attempt to quantify sedimentation rates in known depositional locations. ~~Detailed~~ A detailed evaluation of the radioisotope data from these cores is provided in [Appendix H, Attachment H-5](#). However, generation of a timeline with depth was precluded as these depressions appear to be collecting a complex mix of suspended and bedload sediments with heterogeneous origins. The history of the borrow pits (~~i.e.,~~ the estimated original depths of the borrow pits when excavated in 1988 compared to the observed mudline elevations at the time of sampling in 2007) points to long-term average sedimentation rates in these low-lying portions of the channel of approximately 45 cm/yr (1.5 ft/yr) at RM 10.9 and 30 cm/yr (1 ft/yr) at RM 10.5 (see [Appendix E](#) ~~Appendix H, Attachment H-5~~). Additional empirical evidence on relatively long-term net sedimentation rates is provided by the harbor-wide measured riverbed elevation changes over the seven-year period from 2002 to 2009 (see Map 3.1-67). The net sediment accumulation rates in these upstream borrow pits at RM 10.9 and 10.5 based on these data are estimated to be approximately 41 and 31 cm/yr at RM 10.9 and 10.5, respectively, over this time-frame; ~~this is~~ consistent with the 19-year estimates noted ~~above~~ previously. The borrow pits themselves, spanning the channel in this reach and presenting a relatively larger channel cross-sectional area (because of their greater

Commented [KK103]: Move Appendix H Attachment 5 to Appendix E and reference here.

Commented [GR104]: Appendix H content is being moved to Section 5 based on verbal comments on Section 5 from EPA, callouts will be finalized when Section 5 is finalized

Commented [KK105]: This information is not discussed in Section 5 since it is not discussing nature and extent of contamination. The 3 pages of text in Appendix H can be added to Appendix E.

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depth) than in reaches immediately upstream, provide conditions that promote sedimentation. However, based on the bathymetric survey data, sedimentation rates in this portion of the [Lower Willamette River](#) outside the borrow pits, ~~i.e., on~~<sup>such as</sup> the large shoal that occupies the western portion of the navigation channel from RM 8 to 10, are comparable in scale (31 cm/yr at the maximum shoaling point at RM 9.6, see Map 3.1-10). This shoal area has historically required regular maintenance dredging (see Section 3.2.[3.1.135-2](#)).

The long-term sedimentation rate observations noted above apply to the Study Area channel environment. Based on bathymetric change, SPI data, and limited radioisotope sampling for monitored natural recovery (MNR) assessment (Anchor 2005), nearshore and off-channel areas do not appear to accumulate sediment at these rates. Short-term active sediment deposition and resuspension are indicated by these data sets, likely due in many areas to anthropogenic activity. Seasonal (rainy season) inputs of fine-grained sediments in areas adjacent to the channel are also evident. However, seasonal comparison of surface sediment textures at similar locations in the spring versus the fall suggests that some nearshore deposits can be remobilized over time and dispersed (WEST and Tetra Tech 2009), minimizing net accumulation rates. These observations are supported by the radioisotope data from four nearshore areas in 2004 (Anchor 2005), which show well-mixed surface sediment layers and calculated net sedimentation rates of approximately 1 cm/yr.

~~As illustrated in Map 3.1-12 and noted previously, the FS-HST model prediction of bed elevation change for the high-flow flood scenario depicted in~~<sup>on</sup> Figure 3.1-19 ~~suggests~~<sup>indicates</sup> that the spatial pattern of erosion predicted by the model for the extreme event is generally consistent with measured bathymetric change from 2002 to 2009 under more typical hydrologic conditions. However, in some areas, the magnitude of bed changes during the extreme event is dramatically greater, with erosion or deposition predicted to occur to one or more meters over observed changes (Map 3.1-12).

#### 6.2.1.3.3 Surface Sediment Dynamics

Particles that settle out or move along the bottom are subjected to a wide range of physical, biological, and chemical processes:

- **Sediment mixed-layer turbation** – Biogenic mixing by benthic infauna or bottom-foraging fish can preclude or slow consolidation of surface sediments, as can natural (e.g., wind waves) and anthropogenic (e.g., prop wash) forces. These factors can greatly complicate the spatial and temporal degree of bed erodibility. The SPI survey conducted throughout the [Lower Willamette River](#) in the late fall of 2001 revealed a complex mosaic of surface sediment processes in the top 22 cm of the sediment column (the maximum depth of the SPI images) across the Study Area (SEA 2002b). Areas of fine-grained, low-shear sediments contrasted with coarse-grained, more compacted bottom areas. In the channel environment, these large-scale gradients in gross characteristics

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coincided with and helped first define the hydrodynamic reaches described in Section 3. In some fine-grained areas, infaunal feeding pockets and worm tubes indicated that biogenic activity approached 20 cm depth. In other areas, minimal biogenic mixing activity was apparent. A well-mixed, biologically active zone appears to be on the order of 5 cm in many images, although this varied widely across the Study Area. Many nearshore areas showed steep onshore-offshore gradients in physical and biological conditions as a function of water depth, riverbed slope, and/or the degree of shoreline protection (e.g., embayments, structures). In some areas, layers of freshly deposited sediments exceeding 10 to 15 cm in extent were apparent. This survey was conducted during the onset of the rainy season in late November. Overall, the SPI survey in combination with the bathymetric change data point to a dynamic surface sediment bed in much of Portland Harbor that is subjected to, ~~depending on location and season,~~ physical disturbance in the form of deposition or scour (on a multi-centimeter scale) due to natural and anthropogenic forces, biogenic mixing, and geochemical disturbance factors, such as methane bubble ebullition. Under typical (~~i.e.,~~ non-flood event) flow conditions, ~~however,~~ these disturbance factors appear to be limited to a “maximum” extent of the top 30 cm harbor-wide. ~~Thus, and frequent,~~ widespread physical and biological surface sediment mixing is likely restricted to much shallower depths over much of the Study Area.

- **Long-term sediment burial beneath the mixed layer** – Particles and associated ~~chemicals-contaminants~~ that are advectively transported or buried below the mixed layer (~~less than or equal to 30 cm~~) are permanently removed from the active transport system throughout most of the Study Area. In portions of the navigation channel upstream of RM 10.5 and between RM 5 to 7, erosion of bedded sediments to about 2 m is predicted to occur during 100-yr flood events, but this deep erosion is ~~very~~ limited in areal extent (see Map 3.1-~~89b~~). Consequently, absent anthropogenic disturbance (~~e.g., dredging~~), bedded sediment below 30 cm in most of the Study Area is stable under the long-term hydrologic conditions anticipated for Portland Harbor.
- **Sediment ingestion/uptake by biota** – Filter and deposit feeder organisms may actively or passively ingest particles in suspension or on the sediment bed. High densities of filter feeders can biologically enhance transfer of suspended particles to the sediment bed. Also, ~~chemicals-contaminants~~ associated with ingested particles can enter the food web.

#### 6.2.1.4 Pore Water Physical Transport Processes

~~Chemicals-Contaminants~~ in pore water are subject to diffusive and advective physical transport processes. These mechanisms are discussed in the following subsections.

#### 6.2.1.4.1 Diffusive Transport

Diffusion is the movement of particles or dissolved ~~chemical contaminants species~~ from higher ~~potential energy~~ to lower potential energy (as represented by a difference in concentration in the case of diffusion from the pore water to the overlying water column). This is a spontaneous physical process that requires no additional energy inputs or expenditure. ~~This mechanism~~ It is distinguished from advective transport ~~of chemicals in pore water (described in the following section)~~ in that it only requires ~~no driving force other than~~ a concentration gradient. Diffusive transport acts on any ~~IC~~contaminants in solution and is therefore potentially relevant to all of the combined loading list ~~IC~~contaminants.

Diffusive transport of ~~ICs~~contaminants is being assessed for the Study Area as part of the ~~Fate fate~~ and ~~Transport transport~~ Model ~~model~~ under development for the FS.

#### 6.2.1.4.2 Advective Transport

Advective transport of ~~IC~~contaminants in the sediment/pore water environment refers to the aggregate movement of ~~chemicals contaminants~~ by flow of pore water through the sediments. ~~Flow through the sediments to the water column occurs in the form of groundwater discharge. This mechanism is a~~It represents transport ~~process pathway~~ for ~~chemicals contaminants in the surface sediment/pore water environment~~ to migrate to the water column. ~~As such, it, and~~ is distinguished from the upland groundwater plume loading term, described in Section 6.1.5, ~~which represents advective loading of chemicals from upland groundwater sources to the pore water, sediments, and water column.~~ In ~~some certain~~ parts of the Study Area, both mechanisms are likely occurring simultaneously for ~~chemicals contaminants~~ present in upland plumes and in sediments from other sources. A detailed ~~semi-quantitative~~ estimation of Study Area-wide loading to surface water via groundwater advective transport is presented in Section 6.1.6 along with an analysis of the external loading of surface sediment via advective transport from subsurface sediment.

### 6.2.2 Surface Water Fate and Transport Processes

~~Fate and transport processes for contaminants ICs in surface water are~~ present in the dissolved phase and sorbed to suspended solids. ~~Fate and transport processes for ICs in surface water, including chemical~~ partitioning between surface water, air, and suspended sediment; ~~physical transport of surface water and suspended solids; and physiochemical and biological processes are as~~ described below in Sections 6.2.2.1 through 6.2.2.3.

#### 6.2.2.1 Chemical Distribution between Surface Water and Suspended Sediment

~~A general discussion of chemical partitioning processes that can be used to describe and/or predict the equilibrium distribution of a chemical between water (dissolved aqueous phase) and solid (sorbed to sediment or associated organic matter) was provided previously in Section 6.2.1.1 in the context of chemical distribution between~~

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pore water and bedded sediments. As described in Section 6.2.1.1, the tendency of a chemical contaminant to be associated with suspended solids or dissolved in the water column is described by the  $K_d$  term. For organic compounds, this term describes the combined effect of all possible driving forces affecting distribution between suspended sediment and surface water, including hydrophobic sorption into organic matter on the solid surface, electrostatic attractions of oppositely charged ionic functional groups, and covalent bonding or complexation of ionic organic molecules with reactive surface groups. For nonionic organic chemicals, which include most of the organic chemicals on the IC loading list (PCBs, pesticides including DDX, PCDD/Fs, PAHs, SVOCs, and VOCs), the primary driving force defining the  $K_d$  term is hydrophobic sorption, and  $K_{oc}$  is used to describe chemical behavior as a function of suspended sediment  $f_{oc}$ . As described in Section 5.3 (Figure 5.3-19), suspended solids in the Study Area during high flow conditions exhibit lower relative  $f_{oc}$ , and the highest relative  $f_{oc}$  is observed on suspended solids during stormwater events. For inorganic ICs in surface water (metals), mechanisms controlling partitioning behavior include precipitation/dissolution reactions and sorption/ion exchange processes, and partitioning behavior is not considered to be influenced strongly by suspended sediment  $f_{oc}$ .

Figures 6.2-4 through 6.2-8 present the observed partitioning between surface water (filtered) and suspended sediment for the LWG-collected surface water samples for PAHs, DDX pesticides, PCDD/Fs, PCB homologs, and non-DDX pesticides on Figures 6.2-4 through 6.2-8. Limited site-specific empirical information for arsenic, copper, lead, and mercury is presented for general comparison purposes on Figure 6.2-9. These figures also show the literature  $K_{oc}$  and  $K_d$  values compiled for use in the advective loading assessment (Section 6.1.6). For most chemical groups/contaminants (PCBs, pesticides, PAHs), the observed partitioning between suspended sediment and filtered surface water spans a wider range than the literature  $K_{oc}$  values. The source of this variability is unknown, but may be attributable to disequilibrium/non-equilibrium conditions between surface water and suspended sediment, errors introduced by the estimation method for the  $f_{oc}$  content of suspended sediment (see Appendix E, Section 2.2.1), filtered surface water samples not reflecting truly dissolved concentrations, or a combination of these factors. Based on visual inspection of the information presented on Figures 6.2-4 through 6.2-9, the central part of the range of observed partitioning values corresponds with the CT in the literature values for PCB homologs, PCDD/Fs, and non-DDX pesticides. In contrast, the observed partitioning in the RI data set generally appears to be biased high relative to literature  $K_{oc}/K_d$  for PAHs and, to a lesser degree, DDX pesticides and metals. The high observed affinity of PAHs to organic carbon in suspended sediment could be explained by studies performed by McGroddy et al. (1995, 1996) on partitioning behavior of PAHs in sediments from Boston Harbor, which showed that only a small fraction of PAHs present in bulk sediment from Boston Harbor appeared available for equilibrium partitioning. Both their laboratory studies and *in situ* studies produced, empirically-derived log  $K_{oc}$  values were significantly higher/greater than literature values, specifically for phenanthrene and pyrene. Conversely, their data for PCB congeners appeared to show complete equilibration in both the laboratory and *in situ* studies. The general conclusion made by

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~~McGroddy et al. (1995, 1996) is~~ They concluded that PAH compounds associated with soot particles, ~~as is~~ typical in many coastal and estuarine areas, ~~may be less available to exchange with the pore water than suggested by the literature.~~ ~~McGroddy et al. (1996) also state that in their studies, and that~~ equilibrium partitioning models ~~greatly overestimated both the pore water and the desorption aqueous-phase PAH concentrations, by as much as 100 times in some cases a factor of 100.~~ ~~One implication of this finding on fate and transport modeling could be that~~ Thus, modeled aqueous PAHs concentrations of PAHs will be significantly ~~may~~ overestimated ~~(potentially by up to two orders of magnitude) if they are~~ when based on literature values for  $K_{oc}$  and an assumption of equilibrium partitioning.

#### 6.2.2.2 Physical Transport of ~~IC~~Contaminants in Surface Water

Advection, ~~is~~ the flow of river water in response to gravitational forces, ~~and~~ is the primary mechanism for transport of surface water and its load of dissolved and particle-bound ~~IC~~contaminants. ~~River flow is quantified using Water-water velocity and discharge are used to quantify river flow.~~ Water velocity ~~is depends dependent~~ on the slope, shape, and physical characteristics of the riverbed and has the dimensional units of length/time (e.g., ft/s). Discharge, ~~the represents the~~ quantity of water passing a ~~given specific river location within a specific over a given time interval. It is; is~~ calculated as the average velocity times the cross-sectional area of the river, and has the dimensional units volume/time (e.g., cfs or L/yr). The surface water mass flux of an ~~IC~~contaminant is the product of the ~~chemical~~ concentration and the volumetric flow rate of the river, producing dimensional units of mass/time (e.g., kg/yr), as calculated in Section 6.1.1.1 for surface water load estimates.

The dominant direction of water flow in the ~~Lower Willamette River~~ is downstream, along the hydraulic gradient. However, the flow direction reverses on flood tides during low-flow periods (see Section 3.1.3.2). Upstream flow has been identified as far upstream as RM 12.8 during low-flow conditions (Figures 3.1-13a-h; Caldwell and Doyle 1995).

Lateral and vertical movement of ~~IC~~contaminants in surface water occurs primarily as a result of turbulent (eddy) dispersion (mechanical mixing), and to a lesser extent as a result of mixing/diffusion resulting from ~~chemical concentration~~, thermal, and density gradients. The velocity of river water is greatest near the center of the river and decreases toward the sides and bottom ~~of the river~~. These differences in velocity ~~of different water parcels~~ result in velocity shear, which gives rise to eddies. Eddies ~~are may~~ also ~~be~~ caused by channel irregularities, including structures in the water. These processes serve to mix the water and dilute ~~ICs-contaminant concentrations~~ as they move away from the source. The suspended load of particle-sorbed ~~IC~~contaminants can also decrease due to ~~settling of~~ particles ~~settling out onto the to the~~ riverbed sediment surface.

~~Some~~ sources of ~~IC~~contaminants to surface water (such as industrial point discharges or groundwater plume discharge areas) can result in plume formation as the ~~ICs~~

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contaminants mix with and diffuse into river water flowing downstream. Mixing patterns and plume sizes depend on differences in density between the effluent and the river water; the depth, velocity, and turbulence of the river; and any density stratification of the river itself. Density is a function of the temperature and salinity of the water.

Suspended particles provide an important vehicle for exchange of ~~IC~~contaminants between the sediment bed and surface water. Suspended particles can be derived from mineral sources, including eroded and weathered rock, or from organic sources, such as decaying plant material or plankton. The density of mineral particles is generally 2 to 3 g/cm<sup>3</sup>, whereas the density of organic particles is close to the density of water (1 g/cm<sup>3</sup>). The entrainment and settling of suspended particles are functions of river flow rate, particle size, particle shape, and particle density, as described in Section 6.2.1.3. The sediment-carrying capacity of river water increases with increasing stream flow and turbulence, which vary spatially as well as temporally. Stream flow, turbulence, and TSS loads are greater in areas where the river is narrower (e.g., upstream of RM 10), and throughout the river during high-flow events. Within the water column, suspended particle concentrations of ~~suspended particles~~ generally decrease ~~semi-logarithmically~~exponentially from the riverbed to the water surface. ~~The empirical data set demonstrates these patterns, as described in Section 5.3. As seen in Figure 5.3-13, TSS in surface water across the Study Area increases with increasing flow rate; further, the range of TSS across sampling events decreases~~decreasing by RM 2, where the river turbulence decreases ~~and settling increases~~(Figure 5.3-13).

~~Because of the opposing tendencies of the particles to settle out and the water to entrain them, sediment entrained from the river bottom as bedload may be redeposited on the river bottom downstream. This process, which may disperse IC~~contaminants in the sediment as they are transported downstream with the bedload, ~~however~~However, as discussed Sections 6.1.1.2 and 6.2.1.3, bedload is not considered a major contributor to ~~IC~~contaminant transport in Portland Harbor relative to the suspended solids load.

### 6.2.2.3 Physiochemical and Biological Attenuation Processes in Surface Water

~~In addition to equilibrium partitioning, s~~Once ICs enter surface water, several physical, chemical, and biological processes (in addition to equilibrium partitioning, discussed separately above) can result in transfer of ICs-contaminants found in surface water between abiotic media (water, suspended solids, sediment, and air) or in degradation/transformation reactions of ICs. These processes include chemical precipitation, volatilization, abiotic degradation (chemical reaction or photolysis), and biodegradation. With the exception of volatilization and photolysis, these processes also generally pertain to pore water and sediment interactions, and were previously~~previously~~ discussed in Section 6.2.1.2.

~~Volatilization, is the transfer of ICs-contaminants dissolved in surface water to the atmosphere, is a process that transports ICs out of the water column. Volatilization,~~

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**Commented [awc107]:** There is a difference between "semi-logarithmically" (original text) and "exponentially" (EPA comment). Suggest removing the specific term and simply stating that the concentrations decrease from the riverbed to water surface.

**Commented [KK108R107]:** OK

**Commented [awc109]:** Need to verify when we get Section 5 comments to make sure that the TSS discussions are retained as described in Section 5.

**Commented [KK110R109]:** Yes they are. You may need to update figure reference.

and is ~~an~~ most important factor for small organic molecules, such as VOCs. ~~The extent of volatilization~~ It is an IC depends ~~dependent~~ on water ~~the temperature of the water and air temperature,~~ and is a function of the concentration of the IC dissolved concentration, and in water and its vapor pressure. ~~W-The ater~~ turbulence of the water and wind velocity at the air/water interface ~~also will also affects the volatilization rates and extent of volatilization.~~ Among the ICs on the combined loading list (Table 6.0-1), PAHs (particularly the LPAHs), SVOCs (1,2-DCB more so than BEHP, PCP, or HCB), chlordanes, gamma HCH, aldrin, dieldrin, and all of the VOCs are subject to volatilization from surface water. Volatilization typically decreases with increasing molecular weight. A small fraction of the less chlorinated PCB congeners present in the dissolved form are also subject to volatilization out of the water column.<sup>44</sup> Additionally, various forms of mercury (e.g., ~~elem.ental~~ elemental mercury [Hg<sup>0</sup>] and methylmercury) and lead (e.g., tetraethyllead and tetramethyllead) organolead compounds are may also volatile-volatilize from the water column.

Henry's law constant is used to quantify the equilibrium Equilibrium partitioning of ICs between water-dissolved volatilized phases is defined by the ~~and air~~ Henry's law constant (H<sub>2</sub>), and is

$$H = [\text{conc. in air, mg/L}] / [\text{conc. in water, mg/L}]$$

In this formulation of H, the use of the same units for the concentration of the ICs in air and water results in a unitless constant. Henry's law constant is temperature-specific. Henry's law constants are being compiled for the fate and transport modeling effort under development for the FS.

Photolysis (degradation or transformation reactions ~~that~~ occur in response to absorption of solar energy, and irradiation) can occur either directly or indirectly. Direct photolysis involves the disruption of bonds in the molecule ~~by is the breaking of molecular bonds by~~ electromagnetic radiation, particularly high-energy ultraviolet radiation. ~~Organic compounds that are susceptible to direct photolysis include aromatic compounds and various chemicals with double bonds.~~ Indirect photolysis involves the direct photolytic formation of a reactive species such as a hydroxyl radical or oxygen singlet, which subsequently reacts with an organic ~~compound molecule.~~ Indirect photolysis can involve Examples of indirect photolysis include cleavage of aromatic rings, hydrolysis, hydroxylation, or dechlorination reactions. ~~The products of~~

<sup>44</sup> It should be noted that PCBs are subject to volatilization from surface water (especially the less-chlorinated congeners). A study conducted of the Lower Fox River between Lake Winnebago and DePere, Wisconsin (Steuer 1995) indicated that significant amounts of PCBs could be transferred to the atmosphere. The study predicted that over the course of 25 years, 1,300 kg of PCBs would be transported over the DePere Dam and 200 kg would be emitted to the atmosphere by volatilization. While the findings of this Wisconsin study are not directly applicable to the Study Area, since volatilization loss rates are a function of many site specific conditions including water turbulence, temperature, wind speed, the fraction of PCBs associated with particulate material, etc., it is important to acknowledge this relevant loss mechanism. This mechanism will be considered in the fate and transport modeling work. Volatilization from surface water is discussed further in Section 6.2.2.3.

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**Commented [awc111]:** Retain the list of chemicals that are likely to be influenced by volatilization. Footnote 40 also provided important info about PCBs.

**Commented [KK112R111]:** No. It is obvious that SVOCs and VOCs volatilize. Don't need to restate here.

If the information provided in the footnote is not directly applicable to this site, as stated in the footnote, then it is not important information for this RI.

**Commented [KK113]:** Removed comma.

**Commented [awc114]:** Retain equation definition of H.

**Commented [KK115R114]:** This is a common term that can be looked up in any reference document and is not unique to this analysis. It is not necessary to define it here.

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~~photolytic degradation of ICs are varied and may be either relatively non-toxic or toxic.~~  
The degree to which photolysis occurs is affected by the depth and turbidity of the water, and by the intensity and angle of incidence of light. ~~It. Specifically, among the ICs, photolysis~~ can be significant for aldrin, PAHs (especially LPAHs), PCP, TBT, and organolead compounds. Additionally, ~~ICs associated with (contaminants sorbed to)~~ labile organic carbon can be released to the water column through degradation of the dissolved/suspended organic matter. PCBs and PCDD/Fs are also subject to photolysis in surface water, though the process is considered to be minor for PCBs and is only relevant to PCDD/Fs near the water surface (EPA 1994).

### 6.2.3 Biota-Related Fate and Transport Processes

A number of processes govern how organisms living in the Lower Willamette River are exposed to contaminants and how contaminants are transformed, excreted, or stored in tissue. Organisms living in the Lower Willamette River take up contaminants through physically (e.g., diffusion), chemically, and biologically mediated processes, including transfer of waterborne chemicals across gill structures or other tissues, consumption of prey, or ingestion of sediment. Organisms can modify the chemical burden in their tissues through growth, reproduction, excretion, metabolic transformation, or sequestration. Some chemicals are transferred among organisms through trophic interactions, resulting in increases in concentrations of some chemicals at higher trophic levels.

~~Several of the contaminants (e.g., PCBs, pesticides, PCDD/Fs, and PAHs) that have been measured in invertebrates and fish from the Study Area are and similar~~ hydrophobic ~~and contaminants are~~ likely to be highly-associated with organic materials (i.e., lipids in tissues, dissolved or particulate carbon in the surface water, pore water, and sediment). ~~Exceptions include some VOCs detected in localized areas of groundwater discharges, some more soluble organic compounds (e.g., phenolic compounds, chlorinated benzenes), and some metals, which are more readily found in a dissolved or aqueous phase.~~ However, some metals (e.g., lead and zinc) also tend to associate with organic and inorganic solids because the geochemical properties (e.g., ionic charge) governing their behavior tend to promote sorption.

Once released to the aquatic environment, chemicals enter the food web in a number of ways; ~~the process is not sequential in that all trophic levels can interact with abiotic media.~~ The behavior of chemicals within an aquatic food web is briefly described below.

Primary producers such as phytoplankton and plants take up chemicals ~~mainly primarily~~ through diffusion from ~~the water to which they are exposed, although~~ the lipid content of phytoplankton also serves as a substrate for the partitioning ~~and binding~~ of organic compounds ~~that may be in a dissolved state or bound to colloidal organic carbon.~~ ~~Metabolic byproducts~~ Phytoplankton of phytoplankton metabolic byproducts contribute to the colloidal material in the water column, which can also ~~bind with chemicals~~

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~~dissolved in the water column~~ serve as a binding substrate for dissolved contaminants. These colloidal materials can be directly utilized by bacteria, other phytoplankton, and zooplankton; ~~this serves~~ serving as an additional ~~chemical~~ uptake and transfer mechanism for recycling ~~chemicals-contaminants~~ within the water column food chain. Zooplankton prey upon phytoplankton and other zooplankton, further recycling ~~chemicals-contaminants~~ within the water column. More complex aquatic organisms (e.g., invertebrates and fishes) can take up ~~dissolved-dissolved-~~ or ~~colloidally~~ ~~colloidally~~-bound ~~chemicals-contaminants from in~~ surface water and pore water across gill membranes, skin, and other permeable tissues, such as the mantle in clams (shells, exoskeletons, and scales are less permeable). ~~Hydrophobic chemicals bind to sediment particles because of their organic carbon content.~~ Sediment surfaces may be coated with bacteria and bacterial slimes, natural organic polymers, and other amorphous organic molecules that serve as binding sites. Finer-grained sediments ~~(e.g., silt and clay) have~~ a greater surface ~~area-area-to-~~ volume ratio and ~~may thus therefore~~ have a ~~higher percentage of greater~~ organic carbon ~~content and contaminant concentrations~~(and thus hydrophobic chemical concentrations).

Once sediment or prey is ingested by invertebrates and fishes, ~~the rate of~~ chemical absorption across gut membranes is affected by the size of the molecule (larger molecules ~~being are~~ more difficult to transfer across membranes), concentration gradients between gut content and surrounding tissues, acidity of the gut, and other physical/chemical conditions in the gut. ~~Organisms can react with a~~ Absorbed ~~chemicals-contaminants may undergo through~~ various metabolic processes that ~~result in a change in~~ the chemical structure and properties.

~~Absorbed-Once absorbed,~~ metals that are not excreted may ~~end up be~~ stored in calcium carbonate matrices (invertebrates) or bone (vertebrates); ~~these storage sites which~~ tend to reduce the reactivity of the metal. Organic ~~compounds-contaminants~~ that are not metabolized tend to be stored in organs or fatty tissues, including gametes. These stores can be released within the aquatic and terrestrial food webs when these organisms are ingested by others, upon their death and decomposition, or by transfer to their offspring.

The relative contributions of bedded sediment versus surface water contamination to tissue contamination levels in the Study Area ~~is a fundamental question that is to will~~ be addressed in the FS fate and transport modeling effort. Bioaccumulation modeling ~~is part of that effort, and two kinds of bioaccumulation models are being used~~ to predict chemical uptake by invertebrates and fish ~~is presented in~~ (Windward 2009b). A mechanistic model (Arnot and Gobas 2004) that describes the bioaccumulation of hydrophobic organic ~~chemicals-contaminants~~ is ~~being~~ used to evaluate bioaccumulation of selected ~~contaminants~~ COCs (i.e., total PCBs, TCDD-TEQ, ~~PCB-TEQ~~, and pesticides (including DDX) via water, sediments, and prey. Bioaccumulation of other substances, such as PAHs, is evaluated using a statistical approach based on biota-sediment accumulation factors (BSAFs) or biota-sediment accumulation regressions (BSARs). Under current conditions, the bioaccumulation model ~~preliminarily~~ determined that

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sediments are an important source of benthic invertebrate and fish tissue concentrations for the bioaccumulative ~~compounds~~ contaminants.

~~The bioaccumulation modeling and the overall FS fate and transport modeling effort is ongoing, and quantitative statements about the relative contribution of sediment versus other sources to tissue residues cannot yet be made.~~

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